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A TETRA-CORE STRESS ANALYSIS MODEL

By Alan Lee Dobyns David C. Jack

April 1972

EUSTIS DIRECTORATE U. S. ARMY AIR MOBILITY RESEARCH AND DEVELOPMENT LABORATORY FORT EUSTIS, VIRGINIA

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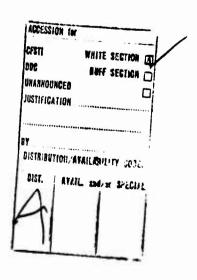
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This study was performed under Contract DAAJ02-71-C-0056 with The Boeing Company, Seattle, Washington. The technical monitor for this contract was Mr. I. E. Figge, Structures Division.

This report contains the results of a study to develop a threedimensional finite-element computer program for "Tetra-Core" elements.

This report has been reviewed by the Eustis Directorate, U.S. Army Air Mobility Research and Development Laboratory and is considered to be technically sound. It is published for the exchange of information and the stimulation of future research.



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A TETRA-CORE STRESS ANALYSIS MODEL

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Ву

Alan Lee Dobyns David C. Jack

Prepared by

The Boeing Company Seattle, Washington

for

EUSTIS DIRECTORATE
U. S. ARMY AIR MOBILITY RESEARCH AND DEVELOPMENT LABORATORY
FORT EUSTIS, VIRGINIA

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ABSTRACT

The results of a five-month analytical program are presented, describing a stress analysis method for a three-dimensional fiber composite structure. The intersections of the elements or webs of this structure form polyhedra in the form of triangular prisms, Tetra-core, and truncated forms of these.

The finite-element model generates the elements and polyhedra automatically with an optimization routine which selects the minimum weight structure that will meet the stiffness and loads requirements.

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FOREWORD

This report was prepared by The Boeing Company, Seattle, Washington, under U. S. Army Contract DAAJ02-71-C-0056 (DA Task 1F162203A17003), and contains an analytical stress analysis model of a three-dimensional fiber composite, Tetra-Core. The contract was started 28 May 1971 and was completed 17 December 1971. Project Engineer for this program was Mr. I. E. Figge, Sr., Structures Division, Eustis Directorate, U. S. Army Air Mobility Research and Development Laboratory.

The authors acknowledge the following Boeing personnel for their support: David L. Beste and Robert Blaisdell.

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INTRODUCTION

This document contains a description of a three-dimensional finite-element stress analysis computer model developed to provide a tool for defining the geometry of Tetra-core structures. The model uses material properties and applied loads in determining the final composite configuration. The primary polyhedron shape is a tetrahedron; one form of the model can produce a Tetra-core, where all elements intersect to form an equilateral tetrahedron.

The objective of the program was to develop a finite-element stress model that would provide a method of analysis of a complex nonhomogenous structure. The model was developed to run on the IBM 360/44. The size of problem that can be run is limited only by the number and size of high-speed discs available.

Tetra-core is a new construction concept developed for use on helicopters, airplanes, and other vehicles where lightweight and damage-tolerant structures are desirable. A typical application is shown in Figure 1. It is made by filament winding fiber glass or advanced composite fibers into an interlocking group of tetrahedrons, as shown in Figure 2. This arrangement appears to give a lightweight, truss-type structure that will be able to sustain damage and still carry a load, because of the high degree of structural indeterminancy. Tetra-core is constructed using a standard filament-winding machine. The fibers are laid in legs oriented in one direction, then the fibers of the next direction are laid over them, thus providing good load transfer between legs. Adhesive is applied to the fiber as it is being wound. The final composite is then ready for the curing process.

Several stress analysis methods have been considered for use with Tetra-core. A simple stress analysis, in which the load in each leg is resolved into its components by a summation of forces, does not account for the structural indeterminancy of Tetra-core, and will probably give conservative results.

The classical laminated plate theory widely used with boron and graphite-layered panels does not apply to Tetra-core since it is not a homogenous material. Each leg of a Tetra-core can be analyzed using laminated plate theory, but a method that accounts for the redundancy caused by the interweaving of the legs should be used in determining the strength of a Tetra-core panel.

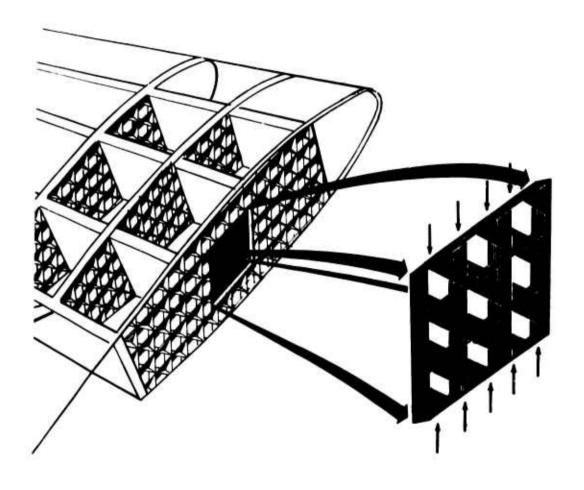


Figure 1. Typical Application of Tetra-Core.

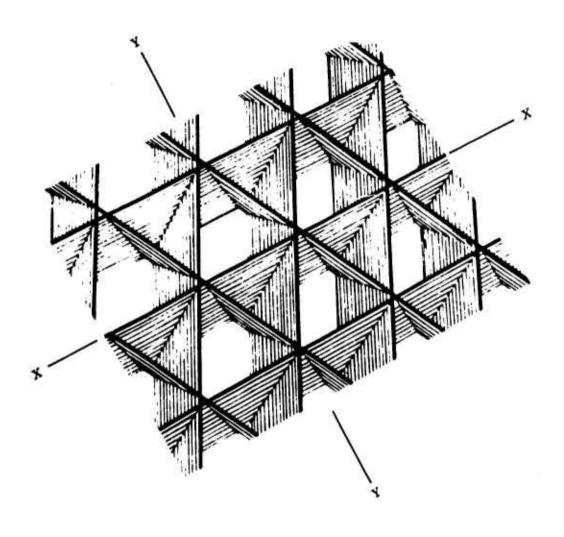


Figure 2. Basic Tetra-Core Element,

The finite-element method was used for this contract, along with an automatic grid generator to automatically idealize the structure and apply loads to it.

Two types of elements are used in this program: a linear strain triangle (6 nodes) for the legs of the Tetra-core, and a constant strain triangle (3 nodes) for the face sheets attached to the core. The linear strain triangle is required for the legs because of the change of strain from tension to compression between top and bottom of the tetrahedron when the panel is subjected to a bending moment. If a constant strain triangular element were used in this strain field, the result would be an averaging of the maximum and minimum stresses since the element would enforce a constant strain over the triangle. A constant strain triangle may be used for the face sheets, however, since the stress field does not contain changes of sign or large strain gradients. This allows a more efficient use of the available core storage space since additional nodes do not have to be generated at the midpoint of each side.

The program has been written to automatically generate finiteelement models for Tetra-core structures made up of flat plates, cylinders, or airfoils. Two basic types of Tetra-core may be generated: a true Tetra-core, where the sides of the Tetra-core meet at a point to form a tetrahedron; and a truncated Tetra-core, in which the tops have been "cut off" of the tetrahedron. The basic tetrahedron geometry can be varied to give a skewed tetrahedron. Self-equilibrating nodal loads are automatically applied to the model based on the input combination of in-plane and out-of-plane loads. Multiple load cases may be run. Boundary conditions (nodal fixities) are applied to the model to hold it from translating or rotating without providing reactions for the nodal loads. This combination of loads and boundary conditions results in the idealization of a coupon of Tetra-core to which loads corresponding to those in some location of a larger structure are applied. A conventional finite-element analysis would be run of the total structure or a part of the structure. Stresses from this run would then be used to apply loads to the Tetracore model corresponding to critical locations on the total structure.

Deflections are calculated using a Choleski triangularization method with substitution to solve the equation [K] $\{\delta\} = \{P\}$. Stresses and margin of safety are calculated at several locations in the linear strain elements. A buckling analysis of a triangle in each leg is done using the buckling equation of an orthotropic rectangle of the same size as a triangle in that leg, since no orthotropic triangle buckling equation are available. An optimization of the Tetra-core geometry can be made

using a steepest descent/side-step method to find a minimum weight design for the given loading conditions. A nonlinear analysis can be run using stress-strain curves of the material in each leg to account for the change in stiffness of each plate element with increasing load. The effect of a hole in a Tetra-core structure can be analyzed. The program will give a zero stiffness to plates connecting to a node at the center of the hole. This causes the loads to be redistributed into plates around the hole, creating the effect of a stress concentration.

The program is now configured for the IBM 360/44, but it can be easily modified to run on any 360. Converting to a CDC 6600 would be more difficult, since direct-access disc drives are required.

Figure 3 shows the elements of the program and the sequence of calculation. Each program element, input and output data, and model usage are described in the following chapters. This includes input format, output format, and a complete listing of the program.

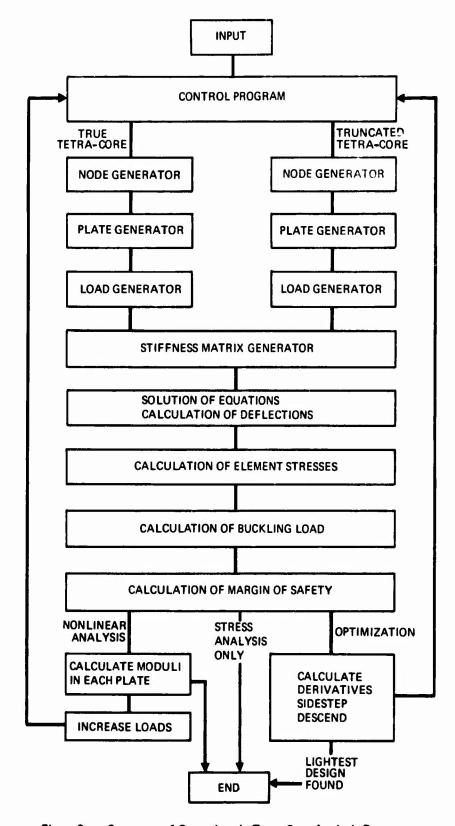


Figure 3. Sequence of Operations in Tetra-Core Analysis Program.

MODEL DESCRIPTION

This program has been written to automatically generate a finite-element model as a series of node points in space and connect them with plate elements into an idealization of a Tetra-core structure (Figure 1). It will automatically apply nodal loads to the model to represent uniform applied loads. Tetra-core flat plates, cylinders, and airfoils can be analyzed. The program uses the finite-element model it has generated to calculate stresses and deflections for each element and each node. A margin of safety is calculated for each plate element. A simple buckling analysis is performed on a plate in each leg. An optimization can be done to determine the least-weight Tetra-core design for a given set of loads. A nonlinear analysis can be performed by inputting a set of material stress-strain curves and applying loads in increments to compute the degradation in each plate element. The effect of a hole in the model can be simulated by giving a low stiffness to the plates connected to a specified node. A diagram of the sequence of computations performed by the program is given in Figure 3.

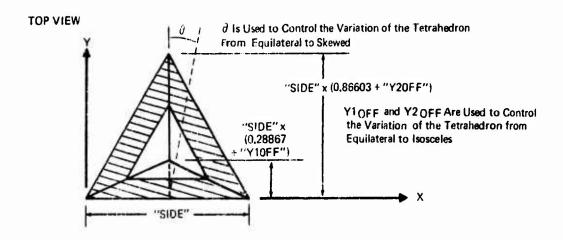
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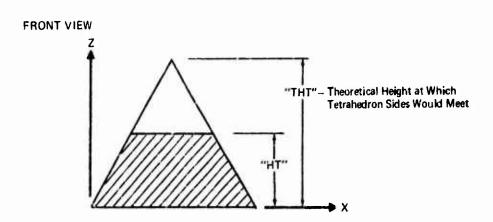
Input data required to run the Tetra-core analysis program has been kept to a minimum to simplify the work required for trade-off studies on the effect of geometry, materials, and other variables. Basic inputs are:

- o Type of analysis to be run--nonlinear, optimization, damage, face sheets, flat plate, cylinder, airfoil, and true or truncated tetrahedron
- Optimization controls
- o Size of specimen to be analyzed and tetrahedron geometry
- o Material data--moduli and Poisson's ratio, allowable stresses, and stress-strain curves
- o Loading to be imposed on the model

Using these data, the program generates a set of node points and plates to model the Tetra-core element and applies the specified loadings as nodal loads.

The basic tetrahedron geometry, shown in Figures 4 and 5, is input as the length of a side, the actual tetrahedron height, the theoretical height at which the apexes would meet, and the offsets which determine the amount of deviation from a true tetrahedron.





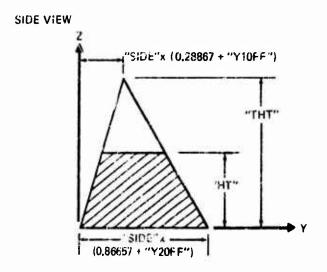


Figure 4. Basic Tetrahedron Geometry,

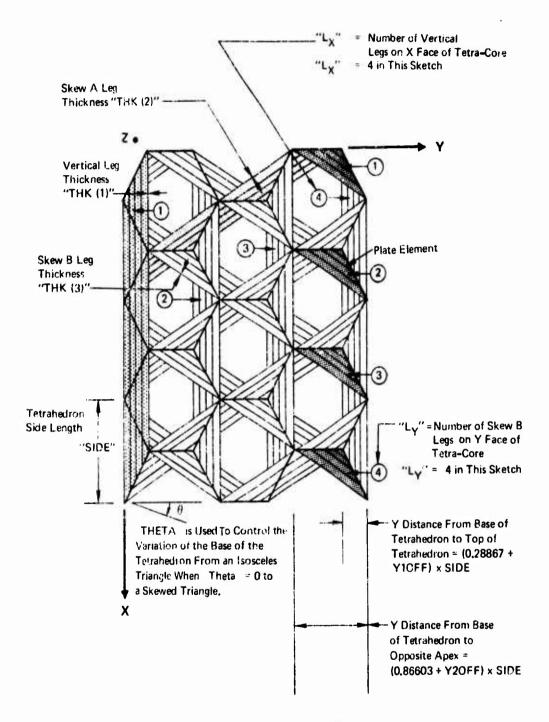


Figure 5. Tetra-Core Input Geometry.

If the actual height and the theoretical height are identical, then a true tetrahedron will result. Two generation options are available in the program: a true tetrahedron and a truncated tetrahedron. The true tetrahedron option should be used whenever possible, since it requires fewer nodal points and plates to be generated, and uses a more efficient nodal numbering system that allows a smaller bandwidth when used in a cylinder or an airfoil. If the theoretical height for a truncated tetrahedron is input as a large number (such as 10 inches), then a three-sided figure with near-vertical walls will result. The truncated Tetra-core model cannot be used to generate a true Tetra-core, since three node points at the top of a tetrahedron would have the same coordinates. would cause a singular stiffness matrix and result in a bad solution. Offsets can be used to change the shape of the figure from a true tetrahedron with angles of 60 degrees, if no offsets are used, to an isosceles tetrahedron of any angle, if offset is used, to a skewed tetrahedron, if θ is used. The number of legs on the X and Y faces (LX and LY) must be even numbers (2, 4, 6, etc.).

Plates to connect the node points and form a Tetra-core model are generated so that the local X-axis is always in the direction of the fibers in that leg, as shown in Figure 6. For sandwich faces, the local X-axis is in the global X direction. Elastic moduli, allowable stresses, and stress-strain curves are input separately for each leg and each face sheet, thus allowing each leg to be a different material if desired. Material stress-strain curves are input only if a nonlinear analysis is run.

Using the input data LX and LY and the geometry, the tetrahedrons are automatically integrated into a Tetra-core node system as shown in Figure 7 for a true tetrahedron and in Figure 8 for a truncated tetrahedron. The node point numbering system is shown for each type. Other examples are shown in Appendix I.

The number of plates generated depends on the input number of legs. Plates are divided into Vertical, Skew A, and Skew B legs by the program. In Figures 9 and 10, the Skew A and B plates generated for a true tetrahedron with 10 legs on the X face and 6 legs on the Y face are shown. Plates are numbered in sequence with plates in the Vertical legs generated first, then plates in Skew A legs, then plates in Skew B legs. When cylinders and airfoils are generated, several additional plates are added to "zip up" the seam and connect the two sides with more Skew A and Skew B plates. Figure 11 shows the Skew A plates making up a cylinder. Face sheets can be automatically added to the true Tetra-core model to give a sandwich element. It was not considered feasible to generate a face sheet for the truncated Tetra-core element due to the complex shapes which would be required.

VERTICAL LEG

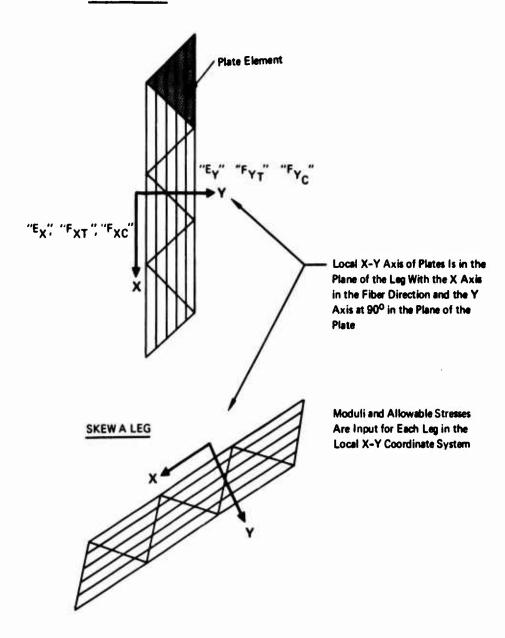


Figure 6. Material Properties Required as Input,

- Nodes Connected by Dark Lines Are on Upper Surface of Plate
- Nodes Connected by Light Lines Are on Lower Surface of Plate

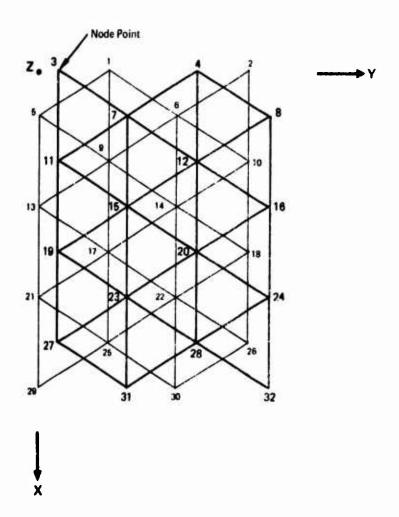


Figure 7. Nodel Numbering System, True Tetra-Core Flat Plate, $L_X = 4$, $L_Y = 4$.

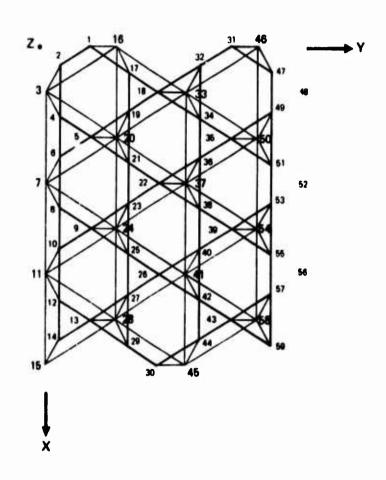
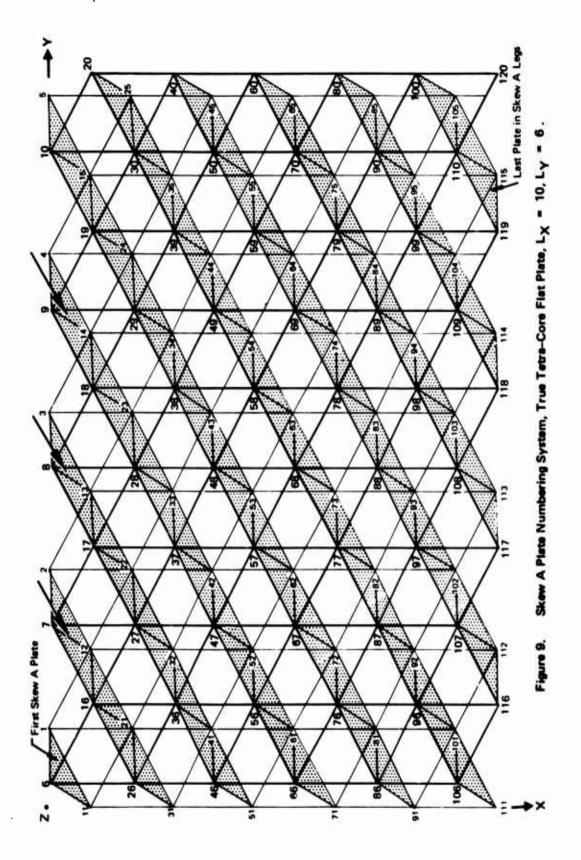
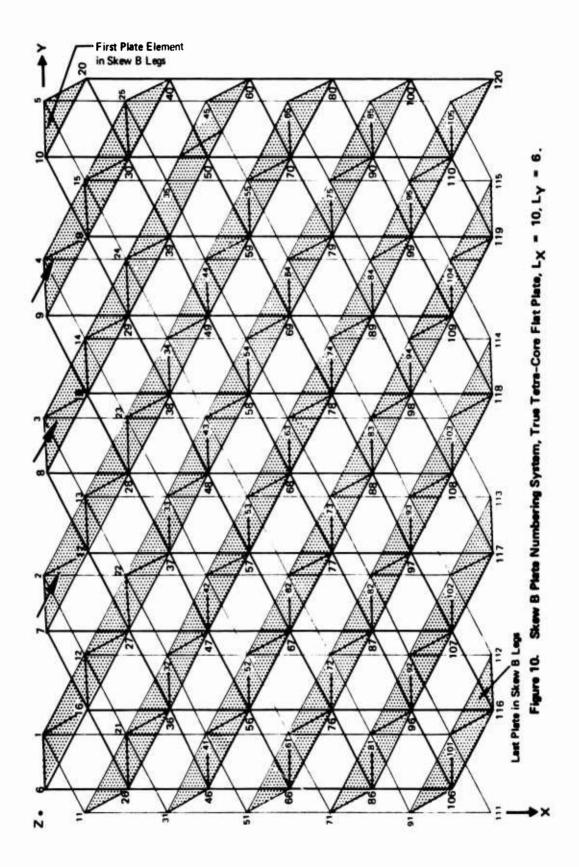


Figure 8. Nodal Numbering System, Truncated Tetra-Core Flat Plate, $L_X = 4$, $L_Y = 4$.





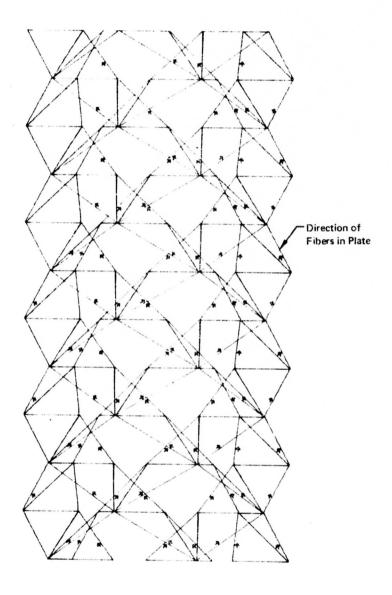


Figure 11. Skew A Plates in Cylinder (Computer Plot).

When generating a cylinder, the program transforms the flat plate Y and Z coordinates into a circular shape with a circumference equal to the Y dimension of the plate. Thus the outside radius of the cylinder is the X dimension of the plate divided by 2π . The airfoil section is generated with an input set of thickness/chord and X/chord ratios to define its shape. Vertical legs are defined to be parallel to the X-axis for cylinders and airfoils to give the maximum bending stiffness.

Node point loads are automatically applied to the finiteelement model based on the input loading conditions. Loads are automatically applied in self-equilibrating sets; i.e., a load applied to one side is reacted by an equal and opposite load on the opposite side. Loading of the linear strain triangle, which is used for the Vertical, Skew A, and Skew B legs, includes a load on the mid-node point between the vertex node points. This load is four times the load at a vertex for a uniform applied load. The vertex node point loadings generated by inputting flat plate end loads in the X and Y directions and shear loads for the true tetrahedron case are shown in Figures For moment loads, the load on the center node 12a, b, and c. point of the triangle side is zero, since the two vertex loads cancel out. Node point loadings generated from input moments in the X, Y, and XY directions are shown in Figures 12d, e, and f for the true tetrahedron. Out-of-plane shear loads can be applied to the model. A moment is automatically applied to react the moment caused by the shear loads as shown in Figures 12g and h for vertical shear loads on the X and Y faces.

The truncated tetrahedron is loaded in a similar manner, except that the lower node points in an edge are given twice the load of the upper node points, since there are twice as many upper node points. The mid-side node point load is four times the upper node point load. More detailed examples of loads applied to Tetra-core flat plates are shown in Appendix I.

The cylinder model can be loaded with moment, torque, and end loads. The input load is used as the total load on the section and is divided among all nodes on that end. The nodal loads generated from an input torsion load on a cylinder are shown in Figure 13.

The airfoil model can be loaded with moment, torque, and end loads in a similar manner to the cylinder. A torsion load applied to a diamond airfoil section is shown in Figure 14.

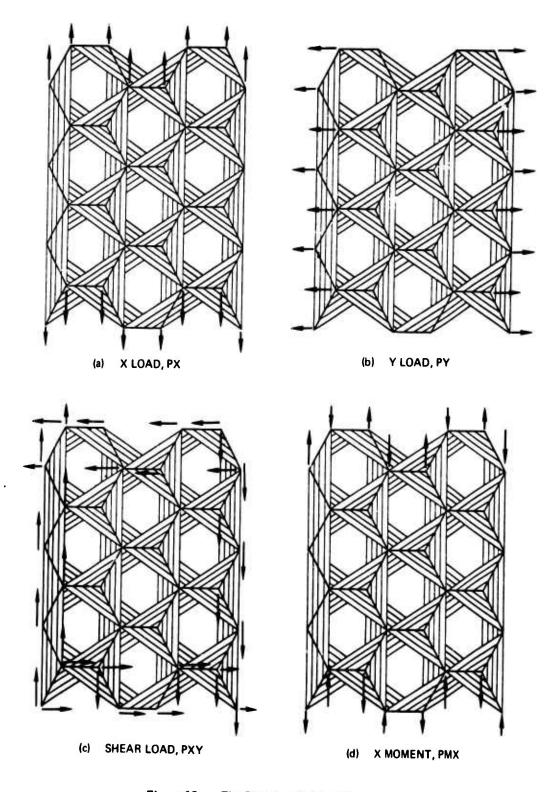


Figure 12. Flat Plate Applied Loads.

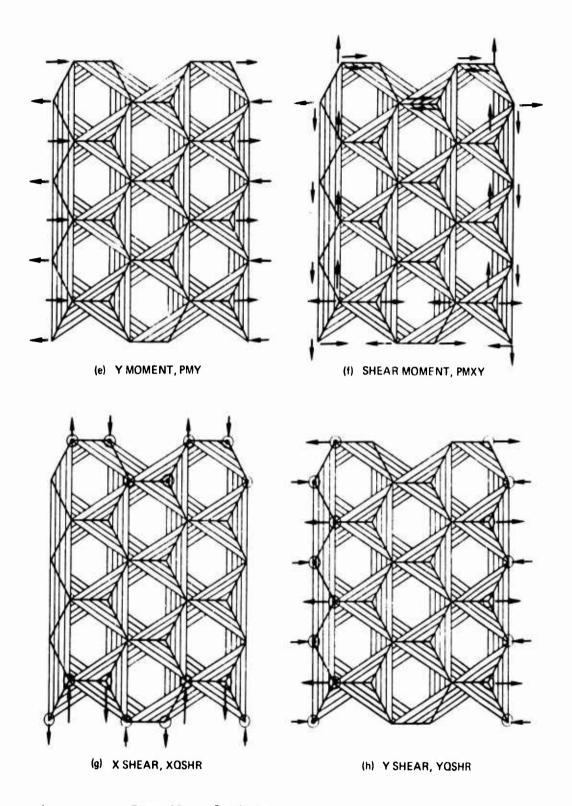


Figure 12. Continued.

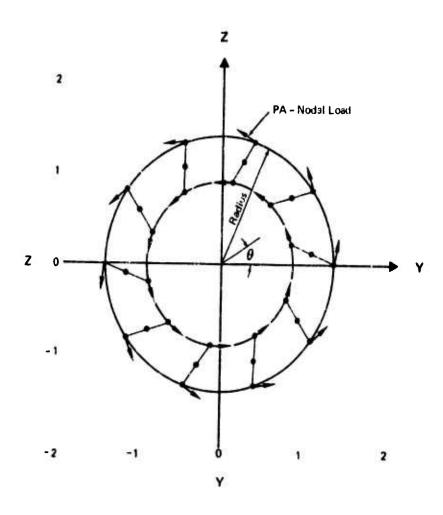


Figure 13. True Tetra-Core Cylinder With Torsion Load Applied, $L_X = 10$.

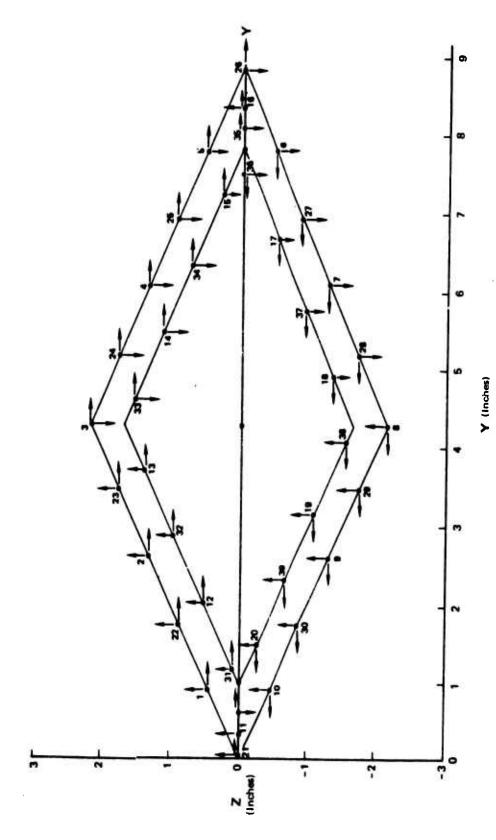


Figure 14. True Tetra-Core Airfoil With Torsion Load Applied, $L_X = 20$.

Loads can be applied to the true Tetra-core specimen being analyzed in any combination (end loads, moments, vertical shears, etc.). The truncated Tetra-core specimen can be loaded with end loads and moments in any combination, but if a vertical shear load is applied, then only certain combinations of loads may be applied at the same time, because of the fixities that are applied to the four sides of the panel to keep the model stable. If no vertical shear loads are applied, the Z-freedom is fixed for all nodes on the panel sides. If a vertical shear is applied on the X face, the side nodes are fixed in the Y direction, which permits movement in the Z direction. A vertical shear load on the Y face causes edge nodes to be fixed in the X direction. Thus, if a load in the Y direction is applied to a panel with a vertical shear load on the X face, the effect of the Y loads will be lost, since they are applied to nodes that cannot move in the direction of the load.

Moment, torsion, and end loads can be applied in any combination to both true and truncated Tetra-core cylinders and airfoils.

The effect of a hole in the Tetra-core can be simulated by giving the plates connected to a node in the center of the panel a zero stiffness. This unloads those plates and transfers the load to the remaining plates, providing the effect of a stress concentration around the hole.

AUTOMATIC MODEL GENERATOR

The Tetra-core analysis program has been written so that a finite-element model will be generated from the input data.

- o Type of Analysis -- Card Type 1
- o Optimization Controls--Card Type 2
- o Geometry--Card Types 5, 6, and 7
- o Material Data--Card Types 4, 8, 9, and 10
- o Loading--Card Types 12 and 13

Flat Plate Nodal Generator

A different nodal generator subroutine has been written for each type of Tetra-core, true and truncated. Subroutines NODGEN and NODGAN are used to generate nodal coordinates for the true and truncated Tetra-core models, respectively. True Tetra-core nodes are generated starting at the Y-axis, with numbers increasing as the X distance increases, as shown in Figure 7. Nodal coordinates (X, Y, Z) on the bottom surface of the Tetra-core at a given X distance are generated first,

using the repeating geometry of Tetra-core, then those on the upper surface. Then the X distance is increased and another cycle of numbering is begun. For the truncated Tetra-core, a different system is used because of its more difficult geometry. Numbering is started at the Y-axis on the first vertical leg, as shown in Figure 8. Node numbering follows the first vertical leg to its end, then switches to the second vertical leg, etc.

Nodal coordinates are generated in order of their appearance on the leg, resulting in a mixing of nodes from the upper and lower surfaces. Nodes in the last vertical leg are generated in the same manner, but every fourth node is unconnected to any other node in the flat plate and must be given fixity in the X, Y, Z directions. If a cylinder or airfoil is generated, these extra nodes are used to connect the two sides of the plate after it has been folded so that the two edges almost meet.

Cylinder Nodal Generator

Cylinders are generated from the flat plate coordinates previously generated. The cylinder is assumed to have a circumference equal to the length of the X face of the flat plate plus the width of one tetrahedron. This gives the cylinder an outside radius that is the circumference divided by 2π . The new Y and Z coordinates of each node are calculated:

$$\theta_i = y_i$$
 flat plate *2 π / circumference
 $y_{cyl} = radius * cos (\theta_i)$
 $z_{cyl} = radius * sin (\theta_i)$

The flat plate is essentially rolled into a cylinder so that the two edges of the flat plate are separated by the width of the one tetrahedron side in the Y direction. The same method is used for both true Tetra-core and truncated Tetra-core. An example of the end of a true Tetra-core cylinder is shown in Figure 13.

Airfoil Nodal Generator

Airfoil nodal coordinates are generated by transformation of flat plate coordinates using a curve-fitting subroutine. The same basic method is used for both true Tetra-core and truncated Tetra-core. The flat plate already generated is essentially folded over to make the upper and lower surfaces of the airfoil. The chord length of the airfoil is half the length of the X face of the original flat plate. The Z coordinate of each node is determined using the curve-fitting subroutine with the input t/C and X/C values. The distance

of a node from the X-axis and the airfoil chord length is used to calculate the X/chord ratio of that node. This X/chord ratio is used in the curve-fitting subroutine to determine the thickness/chord ratio of a point on the surface of the airfoil from the input t/C ratios of the airfoil. The Z coordinate of the node is then the thickness/chord ratio for that point times the chord length. See Figure 14 for an example of the end of a diamond-shaped airfoil generated by the program.

Plate Element Generator

Plate elements are generated automatically to connect the nodes from the nodal generator and form either a true Tetracore or a truncated Tetracore plate. True Tetracore plate elements are generated in subroutine PLATGN, with truncated Tetracore plate elements generated in subroutine PLATGA. The same method is used for both models, but a different subroutine is needed for each, since the nodal numbering sequence is different for each.

Flat Plate

Plate element generation starts with the upper plate in the left vertical leg. Linear strain triangular and quadrilateral plates are used for the Vertical and Skew A and B legs, since they can be expected to encounter compression strain on one side of the leg and tension strain on the other for some load cases. The mid-side node on the outer face of the element is reduced out, since it will not be connected to another element and would cause the merged stiffness matrix to be singular if left in. Two linear strain triangles are added to get the quadrilateral linear strain element used in the truncated Tetracore model. See Figure 24 for the triangle and quadrilateral element node numbering sequence.

Plates are numbered so that the local X-axis of each plate element is in the fiber direction of that leg. Vertical plates are generated by following the vertical leg from top to bottom, then moving to the next vertical leg to the right, and starting at the top again. Each vertical leg has the same number of vertical elements.

Skew plate elements are then generated, starting with the plate in the top left of the flat plate segment, as shown in Figure 9. Skew A legs are generated from upper left to lower right. Generally, each Skew A leg has a different number of plate elements. Next, Skew B linear strain plate elements are generated, starting at the upper right corner of the segment, as shown in Figure 10. Leg generation proceeds from upper right to lower left, with a different number of plate elements in each leg.

If the face sheet option has been specified ("IFACE" = 1 on Card 1), an upper and lower face sheet will be generated for the segment.

Cylinder and Airfoil

A cylinder is generated by folding a flat plate model into a circular cylindrical segment. The two edges of the flat plate facing in the Y direction end up facing each other, separated by the width of a tetrahedron side. The result is a cylinder with a strip missing down the side. Plates are added to connect the two sides of the cylinder, resulting in a complete cylinder, shown in Figure 11. The plates making up a Tetra-core airfoil are generated in the same way, with the flat plate model being folded over into the desired shape and plate elements added to "zip up" the seam where the two ends meet.

Flat Plate Loads

The load generator takes input loads, which are uniform loads (given in either pounds/inch or inch-pounds/inch), and applies them as nodal loads to give an equivalent total load. The same method is used for both true Tetra-core panels and truncated Tetra-core panels. Since a different nodal numbering system was used for each type of Tetra-core, a different subroutine was used for each. Subroutine LODGEN generates loads for the true Tetra-core model, while loads for the truncated Tetra-core model are generated in subroutine LODGAN. When generating nodal loads for the linear strain triangle, the mid-side node must be loaded to give an equivalent uniform load across the element. Theoretically, the load should be divided so that one-sixth of the total load on that plate goes into each corner node and two-thirds goes into the mid-side node¹. This is done automatically in the program.

In the case of a uniform load in the X direction, the input load (pounds/inch) is multiplied by the panel width and divided by the number of nodes on that face, taking account of the fact that some nodes have more load than others.

 $PX = \frac{XLOAD \times XLGT}{NXND}$

where XLOAD = Input load on the X face (lb/in.)

XLGT = Length of X face

NXND = Number of nodes on X face

NXND = 2 x LX + (2 x LX - 1) x 4 for true Tetracore model This gives the X direction load to be applied to each node (PX). Loads are applied to the apex nodes only in the LODGEN and LODGAN subroutines since mid-side nodes are generated in the stress analysis overlay. Nodal loads applied to give the effect of a uniform load in the X direction are shown in Figure 12a for a true Tetra-core flat plate.

Figure 12b shows a load in the Y direction applied to the true Tetra-core panel. The nodal load PY is calculated:

$$PY = \frac{YLOAD \times YLGT}{NYND}$$

where YLOAD = input load on the Y face (lb/in.)

YLGT = Length of Y face (in.)

NYND = 2 x LY + (2 x LY -1) x 4 for a true Tetracore model

LY = Number of legs on Y face

For a uniform in-plane shear load, the loads shown in Figure 12c are applied. The nodal loads are calculated in the same way as for a uniform load in the Y direction, except that a different nodal load (PXYA or PXYB) is used for the X and Y faces.

$$PXYA = \frac{XYLOD \times XLGT}{NXND}$$

where XYLOD = input in-plane shear load (lb/in.)

A moment load in the X direction is applied, as shown in Figure 12d, for a true Tetra-core flat plate. For a moment load, the mid-side nodes are not loaded. Nodal loads are calculated using the moment load per inch, panel width, panel depth, and number of nodes on the side being loaded.

$$PMX = \frac{XMOM \times XLGT}{HT \times LX}$$

where XMOM = Input moment on X face (in.-lb/in.)

A moment load is applied in the Y direction, as shown in Figure 12e. The nodal loads are calculated:

$$PMY = \frac{YMOM \times YLGT}{HT \times LY}$$

where YMOM = Input moment on Y face (in.-lb/in.)

A twisting shear moment is applied as shown in Figure 12f. Nodal loads PMXYA and PMXYB are calculated separately for the X and Y faces of the panel:

$$PMXYA = \frac{XYMOM \times YLGT}{HT \times LY}$$

$$PMXYB = \frac{XYMOM \times XLGT}{HT \times LX}$$

where XYMOM = Input twisting moment (in.-lb/in.)

Out-of-plane shear loads can be applied to flat panels using the input loads XQSHR and YQSHR. A combination of a vertical shear load and a moment load is applied to each loaded end of the panel, as shown in Figures 12g and 12h. The moment load PMX is applied to balance the moment caused by the two vertical shear loads PZ. The resulting set of loads is self equilibrating, so no reactions will result at the fixed nodes.

Calculations of nodal loads for a given X and Y vertical shear are:

$$PZ = \frac{XQSHR \times XLGT}{NXND}$$

$$PMY = \frac{XQSHR \times SIDE \times (LY - 1) \times XLGT}{2 \times HT \times LX}$$

where XOSHR = Input vertical shear load on X face (lb/in.)

$$PZ = \frac{YQSHR \times YLGT}{NYND}$$

$$PMX = \frac{YQSHR \times SIDE \times (0.86603 + Y2 \text{ OFF}) \times (LX-1) \times YLGT}{2 \times HT \times LY}$$

where YQSHR = Input vertical shear load on Y face (lb/in.)

For a true Tetra-core flat plate, these vertical shear loads can be applied with any other loads, such as moment loads and in-plane shear loads; etc. for the truncated Tetra-core flat panel, all loads cannot be applied simultaneously. A vertical shear load YQSHR can be applied with loads and moments on the X face, but cannot be used with in-plane shears or twisting moments or with a moment on the Y face. Similarly, the vertical shear load YQSHR can be applied with loads and moments on the Y face, but not with shear loads or moment loads on the X face. The reason is that the truncated Tetra-core model is

unstable on the edges and must be supported to prevent rigid body rotation in some of the plates. This is done by fixing nodes on the flat plate edges in the Z direction. This allows the flat plate to deflect in the X-Y plane but not in the Z direction. With this fixity, no vertical shear load can be applied to the model because nodes that would be loaded in the Z direction are fixed in that direction and cannot be loaded. When a vertical shear load on the X face is specified, the program will fix the edge nodes in the Y direction, instead of the Z direction, which allows Z nodal loads to be applied. However, loads can no longer be applied to these nodes in the Y direction. Therefore, shear loads and moments on the Y face cannot be applied at the same time that vertical shear loads are applied to the X face. Similarly, if a vertical shear load is applied to the Y face, all edge nodes are fixed in the X direction, and shears and moments on the X face cannot be applied. For the same reason, a twisting shear moment cannot be applied in conjunction with an out-of-plane shear load.

Cylinder Loads Generation

Total end loads, bending moments, and torsion loads are applied to cylinders as nodal loads. End loads are applied to the cylinder by dividing the input X load (in pounds) by the number of nodes on one end of the cylinder, taking into account the fact that some nodes have a larger load than others.

$$PX = \frac{XLOAD}{NXND}$$

where XLOAD = input X total end load on cylinder (lb)

A torsion load is applied to the cylinder as nodal loads tangent to the surface of the cylinder at the node being loaded, as shown in Figure 13. Loads on the inner surface of the cylinder have a smaller load than those on the outer surface by the ratio of their radii.

$$PA = \frac{TORQ}{LX \times (r_A^2/r_A) \times 4}$$

$$PB = \frac{PA r_B}{r_A}$$

where TORQ = Input total torsion load on cylinder (in.-lb)

 r_A = Radius of outer surface of cylinder

 r_n = Radius of inner surface of cylinder

A bending moment is applied to the cylinder as a tension load on nodes below the Y-axis of the cylinder and a compression load on nodes above the Y-axis of the cylinder. The magnitude of the load on each node depends on the distance of that node from the Y-axis. That is, the neutral axis of the cylinder is assumed to be the Y-axis, and a straight-line variation of load with distance from the neutral axis is used.

$$PMX = \frac{XMOM}{SUM}$$

where XMOM = Input total moment on end of cylinder (in.-lb)

$$SUM = 422 (Y_i)^2$$

$$j=1$$

Airfoil Loads Generation

End loads and bending moments are applied to the airfoil in the same way they were applied to the cylinder. A torsion load is applied to the airfoil by applying a load to each end node, in the direction of the airfoil surface at that node, as shown in Figure 14. Since no prediction could be made of the location of the shear center of the airfoil, it was necessary to apply the same total load to each node. This was done instead of ratioing the magnitude of each load by its distance from the shear center, as was done for the cylinder.

Nodal Fixity

In a finite-element analysis, it is necessary to restrain the model from translating or rotating as a rigid body, as this will result in a singular stiffness matrix. This is done by removing the ability of some nodes to move in the X, Y, or Z direction, thus "fixing" them in that direction.

For the Tetra-core flat plate analysis, nodes on the Y face next to the X-axis (nodes 3, 5, 11, 13,..., 29 in Figure 7) are fixed in the Y direction. The first two nodes on the first vertical leg (nodes 3 and 5 in Figure 7) are fixed in the X direction. The lower of the two nodes is fixed in the Z direction. This keeps the model from translating or rotating as a rigid body.

The boundary conditions and system of applied loads used for the flat plate Tetra-core model result in a coupon type analysis. That is, the plate segment being analyzed can be thought of as a small coupon cut out of the larger structure, with loads corresponding to the loads in the larger structure at that point applied to the coupon sides. This allows the use of a coarse-grid finite-element analysis, using a standard finite-element program, to determine the loads at any point in the structure. Tetra-core analyses are then run of several points in the structure, using loads found from the coarse-grid analysis.

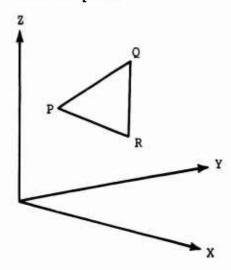
For the truncated Tetra-core model, additional fixities are applied to make the model stable, as discussed in the previous section. Nodes on the panel edge are given fixity in the Z direction to restrain rigid-body rotation of some edge plates. This can be illustrated by looking at the Skew B leg in the upper right corner of a panel (Figure 8). It can be seen that this plate element can pivot about its lower surface node (No. 46) without inducing any in-plane strains into the Skew A or vertical plates supporting it. This results in a singular stiffness matrix, but can be prevented by restraining movement of the edge nodes in the X, Y, or Z direction. If a vertical shear load is applied on the X faces of the panel, then edge nodes are given a fixity in the Y direction instead of the Z direction. If a shear load is applied on the Y faces, then edge nodes are given a fixity in the X direction, as discussed in the Load section.

For Tetra-core cylinders and airfoils, nodes on the end next to the X-axis are given fixity in the X, Y, and Z directions, providing the effect of a cylinder with one end fixed into a stiff foundation.

FINITE-ELEMENT STRESS ANALYSIS

Linear Strain Element

To best derive the stiffness matrix for a linear strain, sixnode triangular element, we first require the geometry expressed in terms of some local in-plane coordinate system.



If
$$X_{QP} = X(Q) - X(P)$$
, etc.,
 $Y_{QP} = Y(Q) - Y(P)$
 $Z_{QP} = Z(Q) - Z(P)$
and $\ell_1 = \sqrt{\left[X_{QP}^2 + Y_{QP}^2 + Z_{QP}^2\right]}$

where $\mathbf{1}_1$ is the length of the line PQ.

Then the direction cosines of PQ are

$$\lambda_1 = X_{QP}/\ell_1 \text{ for } \ell_1 > 0$$

$$\lambda_2 = Y_{QP}/\ell_1$$

$$\lambda_3 = Z_{QP}/\ell_1$$
or $\lambda_1 = \lambda_2 = \lambda_3 \text{ for } \ell_1 \le 0$

The direction cosines of the normal to PQ, through R are obtained as follows:

with RR =
$$\lambda_1$$
 X_{RP} + λ_2 Y_{RP} + λ_3 Z_{RP}

$$X_1 = X_{RP} - \lambda_1$$
 RR

$$Y_1 = Y_{RP} - \lambda_2$$
 RR

$$Z_1 = Z_{PP} - \lambda_3$$
 RR

and
$$\ell_2 = \sqrt{[x_1^2 + y^2 + z^2]}$$

and the direction cosines of the normal are

$$\mu_{1} = X_{1}/\ell_{2}$$

$$\mu_{2} = Y_{1}/\ell_{2}$$

$$\mu_{3} = Z_{1}/\ell_{2}$$

$$P \bigcirc \bigcirc$$

$$R \bigcirc \bigcirc$$

Referring the triangle to local coordinates shown previously and denoting $x_{loc_j} - x_{loc_j}$ by x_{i_j} , we have

$$a_{3} = x_{21} = x_{QP}\lambda_{1} + y_{QP}\lambda_{2} + z_{QP}\lambda_{3}$$

$$-b_{3} = Y_{21} = x_{QP}\mu_{1} + Y_{QP}\mu_{2} + z_{QP}\mu_{3}$$

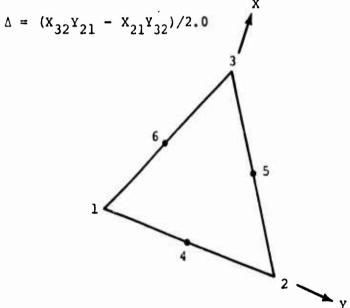
$$a_{1} = x_{32} = x_{RQ}\lambda_{1} + y_{RQ}\lambda_{2} + z_{RQ}\lambda_{3}$$

$$-b_{1} = Y_{32} = x_{RQ}\mu_{1} + Y_{RQ}\mu_{2} + z_{RQ}\mu_{3}$$

$$-a_{2} = x_{31} = x_{RP}\lambda_{1} + y_{RP}\lambda_{2} + z_{RP}\lambda_{3}$$

$$b_{2} = Y_{31} = x_{RP}\mu_{1} + y_{RP}\mu_{2} + z_{RP}\mu_{3}$$

and the area of triangle PQR is



Following the method of Felippa, the stiffness matrix of the six-node linear strain triangle may be expressed as

$$\begin{bmatrix} \mathbf{B}^{\mathrm{T}} \end{bmatrix} \begin{bmatrix} \mathbf{c}_{11} \mathbf{Q} & \mathbf{c}_{12} \mathbf{Q} & \mathbf{c}_{13} \mathbf{Q} \\ \mathbf{c}_{21} \mathbf{Q} & \mathbf{c}_{22} \mathbf{Q} & \mathbf{c}_{23} \mathbf{Q} \\ \mathbf{c}_{31} \mathbf{Q} & \mathbf{c}_{32} \mathbf{Q} & \mathbf{c}_{33} \mathbf{Q} \end{bmatrix} \begin{bmatrix} \mathbf{B} \end{bmatrix}$$

where B relates the strain at any point within the triangle to the nodal deflections,

and Q =
$$\frac{At}{12}$$
 $\begin{bmatrix} 2 & 1 & 1 \\ 1 & 2 & 1 \\ 1 & 1 & 2 \end{bmatrix}$ (3 X 3)

and
$$\begin{cases} \sigma_1 \\ \sigma_2 \\ \sigma_{12} \end{cases} \begin{bmatrix} c_{11} & c_{12} & c_{13} \\ c_{21} & c_{22} & c_{23} \\ c_{31} & c_{32} & c_{33} \end{bmatrix} = \begin{cases} \varepsilon_1 \\ \varepsilon_2 \\ \alpha_{12} \end{cases}$$

Using Hooke's Law to relate stress and strain, C_{23} = C_{32} = C_{31} = C_{13} = 0, and further, C_{ij} is symmetric.

Matrix B may be expressed as follows:

with
$$\phi_{\mathbf{X}} = \frac{1}{2\Delta} \begin{bmatrix} 3b_1 - b_2 & -b_3 & 4b_2 & 0 & 4b_3 \\ -b_1 & 3b_2 & -b_3 & 4b_1 & 4b_3 & 0 \\ -b_1 & -b_2 & 3b_3 & 0 & 4b_2 & 4b_1 \end{bmatrix}$$

and
$$\phi_{Y} = \frac{1}{2\Delta} \begin{bmatrix} 3a_{1} - a_{2} & -a_{3} & 4a_{2} & 0 & 4a_{3} \\ -a_{1} & 3a_{2} & -a_{3} & 4a_{1} & 4a_{3} & 0 \\ -a_{1} & -a_{2} & 3a_{3} & 0 & 4a_{2} & 4a_{1} \end{bmatrix}$$

$$(3 \times 6)$$

then B =
$$\begin{bmatrix} \phi X & 0 \\ 0 & \phi_Y \\ \phi_Y & \phi_X \\ (9 \times 12) \end{bmatrix}$$

So
$$k = \begin{bmatrix} \phi_{x}^{T} & 0 & \phi_{y}^{T} \\ & & & \\ 0 & \phi_{y}^{T} & \phi_{x}^{T} \end{bmatrix} \begin{bmatrix} c_{11}Q & c_{12}Q & 0 \\ c_{12}Q & c_{22}Q & 0 \\ 0 & 0 & c_{33}Q \end{bmatrix} \begin{bmatrix} \phi_{x} & 0 \\ 0 & \phi_{y} \\ \phi_{y} & \phi_{x} \end{bmatrix}$$

which may be expanded to

$$\mathbf{k} = \begin{bmatrix} \phi_{\mathbf{x}}^{\mathrm{T}} \mathbf{c}_{11} \mathbf{Q} \phi_{\mathbf{x}} + \phi_{\mathbf{y}}^{\mathrm{T}} \mathbf{c}_{33} \mathbf{Q} \phi_{\mathbf{y}} & \phi_{\mathbf{x}}^{\mathrm{T}} \mathbf{c}_{12} \mathbf{Q} \phi_{\mathbf{y}} + \phi_{\mathbf{y}}^{\mathrm{T}} \mathbf{Q} \mathbf{c}_{33} \phi_{\mathbf{x}} \\ - \frac{(6 \times 6)}{-} & (6 \times 6) & (6 \times 6) \end{bmatrix}$$

$$\phi_{\mathbf{y}}^{\mathrm{T}} \mathbf{c}_{22} \mathbf{Q} \phi_{\mathbf{x}} + \phi_{\mathbf{x}}^{\mathrm{T}} \mathbf{c}_{33} \mathbf{Q} \phi_{\mathbf{y}} & \phi_{\mathbf{y}}^{\mathrm{T}} \mathbf{c}_{22} \mathbf{Q} \phi_{\mathbf{y}} + \phi_{\mathbf{x}} \mathbf{c}_{33} \mathbf{Q} \phi_{\mathbf{x}} \\ (6 \times 6) & (6 \times 6) & (6 \times 6) \end{bmatrix}$$

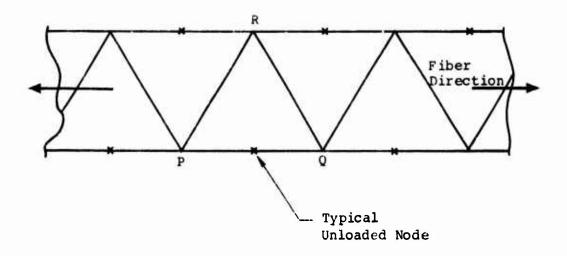
When k is partitioned as above, a small increase in efficiency of computing results as

- 1/ Multiplication by null matrices is avoided
- 2/ Symmetry of the stiffness matrix can be used
- 3/ Storage for a full B matrix is not required.

With

$$\left\{ \begin{array}{c} P_{X_1} \\ P_{X_2} \\ P_{X_3} \\ \vdots \\ P_{Y_5} \\ P_{Y_6} \end{array} \right\} = [k] \begin{cases} U_1 \\ U_2 \\ U_3 \\ \vdots \\ V_5 \\ V_6 \end{cases}$$

In the Tetra-core model, if P-Q is the fiber direction, then the node at the mid-side of P-Q is never loaded or supported out-of-plane.



Upon generating the structural stiffness matrix, this node would, in general, create linear dependence in the stiffness matrix. The linear dependence could be removed by giving the triangle bending stiffness, but this would greatly reduce program efficiency. As the node is never loaded, the simplest and most direct solution is to remove the node at the elemental stage.

This is done as follows: The node to be removed corresponds to node 6. So, when k is rearranged so that it corresponds to the usual order, the rows and columns are rearranged so that those corresponding to U_6 , V_6 occur at the bottom and right-hand side of the matrix.

and the matrix is partitioned so that

$$\left\{ \frac{P_{1-5}}{P_{6}} \right\} = \left[\frac{K_{11}}{K_{21}} - \frac{1}{1} - \frac{K_{12}}{K_{22}} \right] \left\{ \frac{U_{1-5}}{U_{6}} \right\}$$

$$\left\{ \frac{P_{1-5}}{P_{6}} \right\} = \left[\frac{K_{11}}{K_{21}} - \frac{1}{1} - \frac{K_{12}}{K_{22}} \right] \left\{ \frac{U_{1-5}}{U_{6}} \right\}$$

$$\left\{ \frac{P_{1-5}}{P_{6}} \right\} = \left[\frac{K_{11}}{K_{21}} - \frac{1}{1} - \frac{K_{12}}{K_{22}} \right] \left\{ \frac{U_{1-5}}{U_{6}} \right\}$$

PX6, PY6 are each zero, so

$$\{P_{1-5}\} = [K_{11}] [U_{1-5}] + [K_{12}] [U_{6}]$$

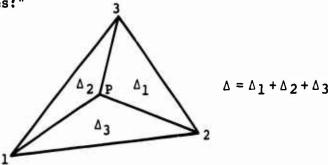
$$\{0\} = [K_{21}] [U_{1-5}] + [K_{22}] [U_{6}]$$

i.e.,
$$\{P_{15}\} = \{K_{11}\} - \{K_{12}\} [K_{22}]^{-1} [K_{21}] \quad U_{1-5}$$

and we have the stiffness of the element expressed in terms of the five remaining nodes as

$$[K_{11}] - [K_{12}]^T [K_{22}] [K_{21}] = K_{RED}$$

In calculating the stress matrix, as in Reference 1, we define "area coordinates:"



$$\xi_1 = \frac{\Delta 1}{\Delta}$$
; $\xi_2 = \frac{\Delta 2}{\Delta}$; $\xi_3 = \frac{\Delta 3}{\Delta}$

Coordinates ξ_i uniquely determine location P, and the stress matrix is

$$\begin{cases}
\sigma_{1} \\
\sigma_{2} \\
\tau_{12}
\end{cases} =
\begin{bmatrix}
c_{11} & c_{12} & 0 \\
c_{21} & c_{22} & 0 \\
0 & 0 & c_{33}
\end{bmatrix}
\begin{bmatrix}
\xi_{i} & 0 & 0 \\
0 & \xi_{i} & 0 \\
0 & 0 & \xi_{i}
\end{bmatrix}
\begin{bmatrix}
B
\end{bmatrix}
\begin{cases}
U_{1} \\
V_{1}^{1} \\
U_{2}^{2} \\
V_{2}^{2} \\
\vdots \\
U_{6}^{6} \\
V_{6}^{6}
\end{cases} i = 1 \rightarrow 3$$

In the program this has been broken down into a more efficient form as

$$\begin{cases}
\sigma_{1} \\
\sigma_{2} \\
\tau_{12}
\end{cases} = \begin{bmatrix}
c_{11} & c_{12} & 0 \\
c_{12} & c_{22} & 0 \\
0 & 0 & c_{33}
\end{bmatrix} \begin{bmatrix}
\langle \psi_{X} \rangle & 0 \\
1 \times 6 & 0 \\
0 & \langle \psi_{Y} \rangle \\
1 \times 6 & 0 \\
\langle \psi_{X} \rangle & \langle \psi_{X} \rangle \\
1 \times 6 & 0 \\
\langle \psi_{X} \rangle & \langle \psi_{X} \rangle \\
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1 \times 6 & 0 \\
\langle \psi_{X} \rangle & \langle \psi_{X} \rangle \\
1 \times 6 & 0 \\
\langle \psi_{X} \rangle$$

where

$$\begin{array}{l} <\psi_{X}> \; = \; \frac{1}{2 \mathtt{A}} \; \; <(4 \xi_{1} - 1) \, b_{1} \, , \; \; (4 \xi_{2} - 1) \, b_{2} \, , \\ (4 \xi_{3} - 1) \, b_{3} \, , \\ 4 \, (\xi_{3} b_{2} + \xi_{2} b_{3} \, , \\ 4 \, (\xi_{1} b_{3} + \xi_{3} b_{1})> \end{array}$$

$$\langle \psi_{Y} \rangle = \frac{1}{2A} \langle (4\xi_{1}-1)a_{1}, (4\xi_{2}-1)a_{2}, (4\xi_{3}-1)a_{3}, 4(\xi_{2}a_{1}+\xi_{1}a_{2}), 1X6$$

$$4(\xi_3 a_2 + \xi_2 a_3), 4(\xi_1 a_3 + \xi_3 a_1) >$$

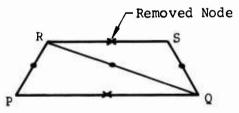
Multiplication by the C matrix is carried out after element strains have been determined in subroutine DEFL. The sixth node is reduced out of the stress matrix in the same manner as for the stiffness matrix.

$$[S T_{RED}] = [S T] - [S T_{12}]^T [K_{22}]^{-1} [K_{21}]$$

The stress matrix is determined for four locations in the linear strain triangle: Nodes P, Q, and R and the triangle centroid.

Quadrilateral Element

The linear strain quadrilateral element used in the truncated Tetra-core model is formed by adding two linear strain triangles from which the sixth nodes have been removed.



The mid-side node between R and Q is then reduced out as the top and bottom mid-side nodes had been, since it is never loaded or attached to another element. This results in a six-node linear strain quadrilateral. The stress matrices are similarly formed, with stress now being computed at the four corner nodes.

Constant Strain Triangle

The constant strain triangle used for face sheets is generated using the method of Przemieniecki³. The local in-plane coordinate system set up for the linear straing triangle

with $a_1 = X_{32}$, etc., is used. A [B] matrix relating strains within the element to deflections of the node points is defined.

$$[B] = \frac{1}{2\hbar} \begin{bmatrix} b_1 & 0 & b_3 & 0 & b_2 & 0 \\ 0 & a_1 & 0 & a_3 & 0 & a_2 \\ a_1 & b_1 & a_3 & b_3 & a_2 & b_2 \end{bmatrix}$$

The stiffness matrix [K] is then

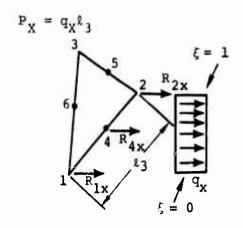
$$(K) = \sum_{i=1}^{N} [C] [B] dV$$

$$[K] = \Delta t [B]^T [C] [B]$$

and

Consistent Surface Loads Vector

The uniform in-plane loading acting on an edge of the linear strain triangle must be resolved into stress resultants acting at the two corner nodes and the midpoint. The derivation shown here is from a document by Tocher. The total force on the plate is



The equilibrium equations can be written using virtual work as

The virtual deflection $\delta U(\xi)$ can be obtained as (Ref. 1)

$$\delta U = \langle \delta U_1 \ \delta U_4 \ \delta U_2 \rangle$$

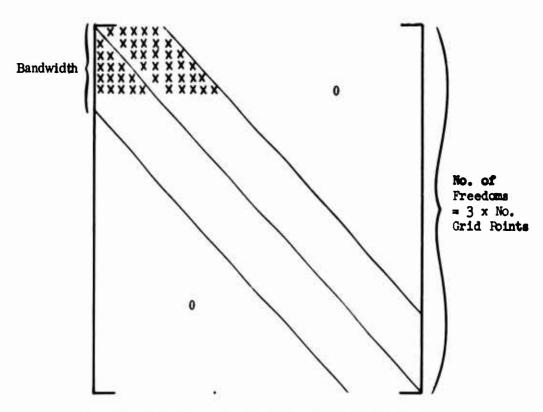
$$\begin{cases} 1 - 3\xi + 2\xi^2 \\ 4\xi - 4\xi^2 \\ 2\xi^2 - \xi \end{cases}$$

Thus

and the load distribution is simply two-thirds of the load through the middle node and one-sixth through each end node.

Calculation of Deflections

A solution of the equation $k\Delta = P$ is used to calculate deflections (Δ) from the stiffness matrix (k) and the loads vector (P). A banded form of the stiffness matrix is assumed, in which only the nonzero portion of the stiffness matrix, which is inside the bandwidth, is stored (Figure 15). The stiffness matrix is stored in blocks on the disc, with each block being three rows. Thus, only one block is in core at any one time during generation of the stiffness matrix. Each elemental stiffness matrix is generated and added into the overall stiffness matrix using the direct stiffness method. The location of the block which each member of the elemental stiffness matrix is added into is determined from the nodal connectivity of that element. Each element will add its stiffness to several blocks, thus requiring the ability to turn the needed block by means of a direct-access disc.



Core Storage Requirement = Bandwidth x No. of Freedoms

Figure 15. Structural Stiffness Matrix.

Once the stiffness matrix has been formed, it is reduced to a triangular form using the Choleski methods³. This results in all members of the matrix below the diagonal being zero.

$$\overline{k}_{ii} = k_{ii} - \sum_{r=1}^{i-1} \overline{k}_{ir}^{2} \qquad i = 1, n$$

$$\overline{k}_{ij} = k_{ij} - \sum_{r=1}^{j-1} \overline{k}_{ir} \overline{k}_{rj} / \overline{k}_{jj} \qquad i < j$$

$$\overline{k}_{ij} = 0 \qquad i > j$$

where n is the order of the stiffness matrix. The loads vector is modified by the same factors that were used to triangularize the stiffness matrix. With the triangularized form of the stiffness matrix, the deflections of the last node can be calculated

$$\overline{k}_{1n}\Delta_1 + \overline{k}_{2n}\Delta_2 + \cdots + \overline{k}_{nn}\Delta_n = P_n$$

where k is the triangularized stiffness matrix.

Since
$$\overline{k}_{ln}$$
, \overline{k}_{2n} ... $\overline{k}_{n-1,n} = 0$

$$\Delta_n = P_n / \overline{k}_{nn}$$

Using this method, $^{\Delta}{}_{n-1}, ^{\Delta}{}_{n-2}, \ldots^{\Delta}{}_{1}$ can be solved, in turn, by back substituting the previously found deflections into the calculations for the next higher row. For example, $^{\Delta}{}_{n-1}$ can be calculated

$$\bar{k}_{n-1,n-1}^{\Delta}_{n-1} + \bar{k}_{n,n-1}^{\Delta}_{n} = P_{n-1}$$

where k_{n} had been calculated in the step before and

$$K_{n-1,n-1}$$
, $K_{n,n-1}$ and P_{n-1} are known

This method of calculating deflections has proved to be much faster than the traditional method of inverting the stiffness matrix and multiplying $k-l_p$ to get Δ . The method allows a large problem to be solved on a computer with small core space,

since only three rows and three columns of the stiffness matrix are in core at one time during solution. However, it requires many accesses to the disc during a solution, since rows and columns must be shuffled in and out of core many times. On the 360/44 at Fort Eustis, the disc space available is the limiting factor on the size of problem that can be run.

Fixities are applied to the stiffness matrix by multiplying the diagonal term of the row to be fixed by a factor of 10¹². This has the effect of giving that freedom a much greater stiffness than other freedoms, effectively fixing it. This is equivalent to putting a one on the diagonal and setting elements of that row and column to zero, but it is more efficient.

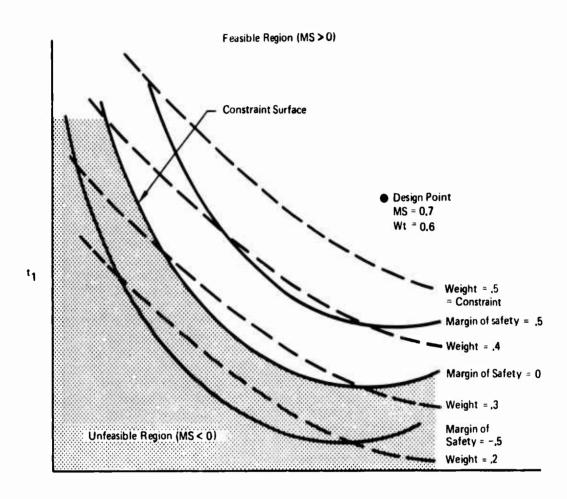
OPTIMIZATION

A modified form of the steepest-descent optimization method is used to find a minimum weight Tetra-core design for a given set of loads⁴. Variables that can be optimized are tetrahedron geometry (side length, height, theoretical height, Yl offset, Y2 offset, THETA) and the thicknesses of each leg. See Figures 4 and 5 for a description of the tetrahedron geometry. Optimization controls have been set up so that any combination of variables can be held constant during optimization. Also, any three variables can be forced to be the same during optimization; i.e., Vertical and Skew A and B legs can be kept to the same thickness during optimization. Input Card Type 2 is used to specify which variables will be optimized.

The optimization method will be illustrated using a twodimensional example so that the variation of design variables can be plotted graphically. For the example, assume that a Tetra-core run is being made in which all variables are held constant except for the vertical leg thickness and the skew leg thickness, and that the Skew A and B legs will have the same thicknesses. Using this model, a design space can be plotted for a given set of loads in which the X and Y axes correspond to the variation in vertical and skew leg thicknesses as shown in Figure 16. For any point in the design space, the Tetra-core element will have a given weight and margin of safety. A series of curves of constant weight and another series of curves of constant margin of safety can be drawn as shown in the figure. The margin-of-safety curve for a margin of safety of zero is used to divide the design space into a feasible region, in which the margin of safety is greater than zero, and an unfeasible region, with a margin of safety less than zero. optimization program does not know the location or shape of these curves, except for the location of the present design, but it can calculate the gradient of these curves; that is, the direction of travel in design space which will result in the maximum change in either the weight or the margin of safety. The gradient can be calculated by changing each variable, by a small amount, while holding all other variables constant and calculating the weight and margin of safety for the new design point. This is done separately for all variables, resulting in a set of partial derivatives:

$$\frac{\partial MS}{\partial t_{i}} = \frac{MS_{i} - MS_{BASE}}{t_{i} - t_{iBASE}} \qquad i = 1, N, N = No. of variables$$

$$\frac{\partial WT}{\partial t_{i}} = \frac{WT_{i} - WT_{BASE}}{c_{i} - t_{iBASE}} \qquad i = 1, N$$



^t2

Figure 16. Design Space.

where ${
m MS}_{
m BASE}$, ${
m WT}_{
m BASE}$, and ${
m t}_{
m iBASE}$ refer to the design point at the start of calculation of derivatives, before changing any thickness. Then these sets of partial derivatives can be used to calculate the required gradients.

$$\beta_{i_{k}} = \frac{\partial MS_{k}}{\partial t_{i}} / \sum_{j=1}^{n} (\frac{\partial MS_{k}}{\partial t_{i}})^{2}$$
 i = 1,N

$$k = 1,M$$

$$M = number of constraints$$

and

$$\alpha_{i} = \frac{\partial WT}{\partial t_{i}} / \sum_{j=1}^{n} (\frac{\partial WT}{\partial t_{i}})$$
 $i = 1, N$

These gradients are actually the direction cosines of the normal to each curve at that design point.

For example, assume that the starting design point is chosen at Point A in Figure 17 and that the weight and margin of safety have been calculated for this point. As yet, no partial derivatives have been calculated, so the thickness will be decreased in steps in the same ratio to get to the constraint curve (MS=0). In the general case, the program changes only the thicknesses in first reaching the constraint curve, leaving the side, height, etc., dimensions constant. Then the program calculates the gradients of the weight and the margin-of-safety curves as previously described. There is a different margin of-safety curve for each leg, since changing the thickness of one leg will change the margin of safety in that leg more than in another leg. This requires the calculation of a gradient of the margin of safety in each leg, as shown in Figure 18. A composite constraint gradient is computed using the gradient to each constraint curve and the margin of safety in each leg.

$$\{\gamma\} = \{\beta_1\} (1-MS_1)^{15} + \{\beta_2\} (1-MS_2)^{15} + ... + \{\beta_M\} (1-MS_M)^{15}$$

The proportion in which each constraint gradient is added into the composite constraint gradient is determined by the distance of the design point from that constraint.

In Figure 18 for case A, in which the design point is at the intersection of two constraints, each gradient is added in equally, resulting in a new direction. In case B, in which the design point is on one constraint and far from the other, the gradient of the constraint which the design is on will have the major influence on the new direction. This composite

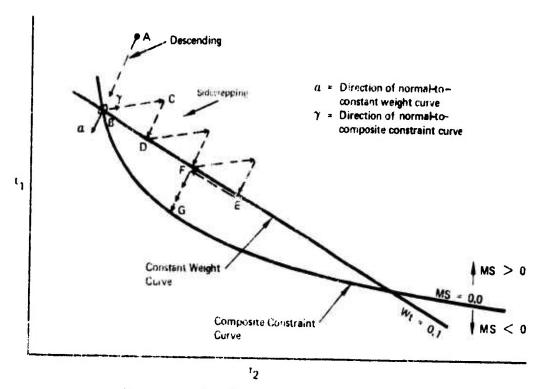


Figure 17. Two-Dimensional Optimization Example.

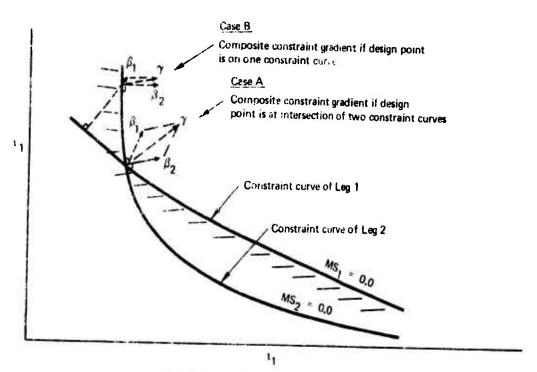


Figure 18. Composite Constraint Gradient.

constraint gradient is used by the program to make a step in the direction of maximum change in margin of safety, to Point C in Figure 17. The step is calculated:

$$t_{i+1} = t_i + \gamma_i x$$

where X is the step length. Next, the design variables are decreased along the gradient to the constant weight curve to reach the starting weight (Point D). This cycle B-C-D results in a sidestep away from the constraint curve in the direction of maximum increase in margin of safety, while maintaining a constant weight. The margin of safety is calculated at Point D. If the margin of safety is now greater than it was at Point B, then the bucket in the curve has not been reached and another sidestep is taken (D-F). Margin of safety is recalculated and another sidestep is taken (F-E). When the margin of safety is calculated at Point E, it is found to be less than at Point F. Therefore, the bucket in the constraint curve has been passed and a backward step is taken (E-F). Now the variables are decreased along the gradient to the constant weight curve (this is known as the direction of steepest descent) in steps, to reach the constraint curve (Point G). This series of operations (B-G) is called one cycle in the program. At the completion of one cycle (Point G), gradients are recalculated, the step length is reduced, and another series of sidesteps is begun. The number of optimization cycles to be run is input as "NSTP" on card type one. If a zero or blank is input for "NSTP," the program will set "NSTP" = 3. If the program makes ten sidesteps (B-D, D-F, etc.) without reaching the bucket, the program will descend to the constraint curve, recalculate gradients, and begin sidestepping again with a larger step length to reach the optimum in a fewer number of steps. At the end of each cycle (B-G), the weight of the design is checked. If the new weight is less than the weight at the end of the previous cycle, the variables are stored as a new optimum design. Generally, the program will find a series of designs, each lighter than the previous one. However, it is possible that the optimum will be found on the first or second cycle, and the rest of the cycles will result in heavier designs. This makes it necessary to store the lightest design found, since the program may not return to it.

In general, the constraint and constant weight curves are not two dimensional but are N-dimensional hypersurfaces, N being the number of variables. Figure 19 illustrates the set of constraint surfaces for a three-dimensional problem. Generally, several optimum designs will be possible for a given set of variables, and the program will only find the one that is

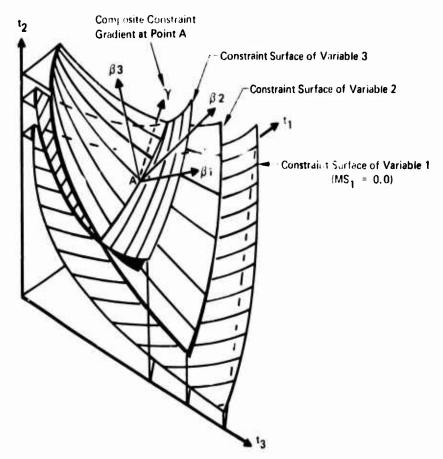


Figure 19. Constraint Surfaces of Three-Dimensional Problem.

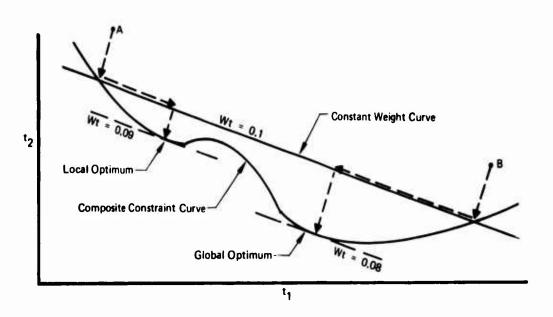


Figure 20. Global Optimum.

nearest to the input starting design. In Figure 20, if optimization is begun from Point A, an optimum design with a weight of 0.9 pound/square inch will be found, while starting at Point B will result in a weight of 0.08 pound/square inch. Thus it is necessary to make several optimization runs to determine if the "global" or most optimum design has been found.

The steepest descent/sidestep optimization method can be thought of as a way of getting down a mountain pass to the lowest altitude possible. The sides of the mountain represent constraints, and lines of constant altitude represent lines of constant weight. Once a given altitude has been reached by the design point, the altitude cannot be increased—only decreased.

A plot of the weight versus number of cycles for a typical case is shown in Figure 21. For this run a true Tetra-core flat plate was optimized with the side length held constant, the height and vertical leg thickness allowed to vary, and the Skew A and B leg thicknesses held the same. The largest change in weight was in the first cycle, indicating that the optimum region was found quickly, with the rest of the cycles being taken up with searching the immediate region to find the exact location of the optimum.

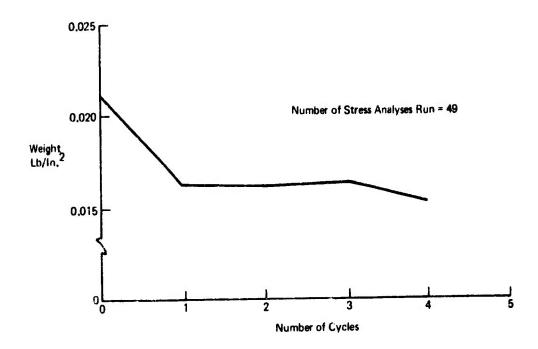


Figure 21. Example of Tetra-Core Optimization.

NONLINEAR ANALYSIS

An analysis of the effect of the nonlinear stress-strain behavior of the fiber-glass-epoxy composite used in Tetra-core legs can be conducted with this program. Stress-strain curves of the composite, determined by cutting out legs from the panel and testing them separately, are used as input, as is the loading increment to be applied5. First, the program calculates the stresses and strains in each plate, using the tangent moduli and Poisson's ratios input on Card Type 4 and the loads input on Card Types 12 and 13. Then, a new modulus is calculated for each plate, using the stress-strain curves input for each leg and the strains already calculated. As an example, suppose that the strain in the fiber direction (ϵ_1) of one plate had been + 0.0011 at the end of the first load increment, and that the tension stress-strain curve shown in Figure 22 had been input on Card Type 8 as a series of points for the leg which this plate was in then the program takes the strain of 0.0011 and uses a linear curve fitting method to determine the stress corresponding to the given strain. This stress is used to compute a secant modulus to be used by the plate for the second iteration:

$$E_{\text{sec}} = {}^{\mathfrak{I}} 1^{/\epsilon} 1$$

The secant modulus is determined in the longitudinal, transverse, and shear directions from the stress-strain curves input for those directions. A different stress-strain curve is input for tension and compression for the longitudinal and transverse directions, and the correct one is chosen by referring to the sign of the strain in that direction.

Once moduli and Poisson's ratio in each plate have been determined, the loads are increased by the amount input on Card Types 12 and 13 and a new stress analysis is run. Then the stresses and strains in each plate are used to determine a new set of secant moduli and Poisson's ratio for each plate. The loads are increased again and the stresses are recalculated. This loading in increments continues until the number of steps specified on Card Type 1 as "NSTP" are completed. As the loads are increased, some plates will reach their failing strain in some direction. When this happens, the secant modulus in that direction is reduced to a small number, which unloads that plate without allowing the nodes to which it is attached to become free, as would happen if the modulus were reduced to zero. On the next loading increment, strains in the adjoining plates will be higher than would be caused by the increase in end load only, since the loads in the failed plate have been dumped into them. This will result in the failure of more plates in

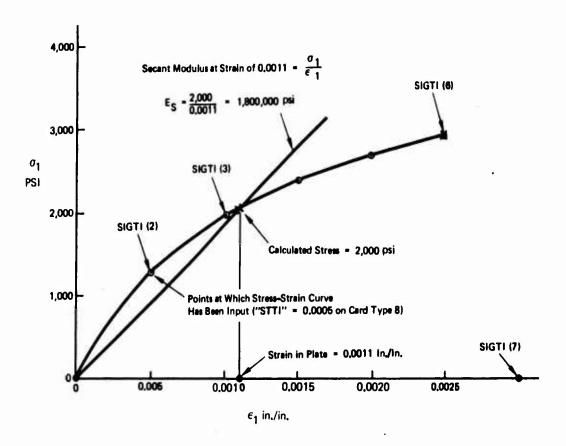


Figure 22. Method Used To Determine Moduli for Plastic Analysis.

the same area. Finally, enough plates will have failed that there is no load path through the structure, resulting in complete failure. Figure 23 illustrates a nonlinear analysis run on a Tetra-core panel with several plates removed to simulate the effect of a hole. Initially, all plates except those that have been removed are intact, but as the load is increased, several plates around the hole fail; and finally at a load of 3,000 pounds/inch, total failure occurs. If a lower increment of load had been used, the spread of failure around the hole would have been more visible.

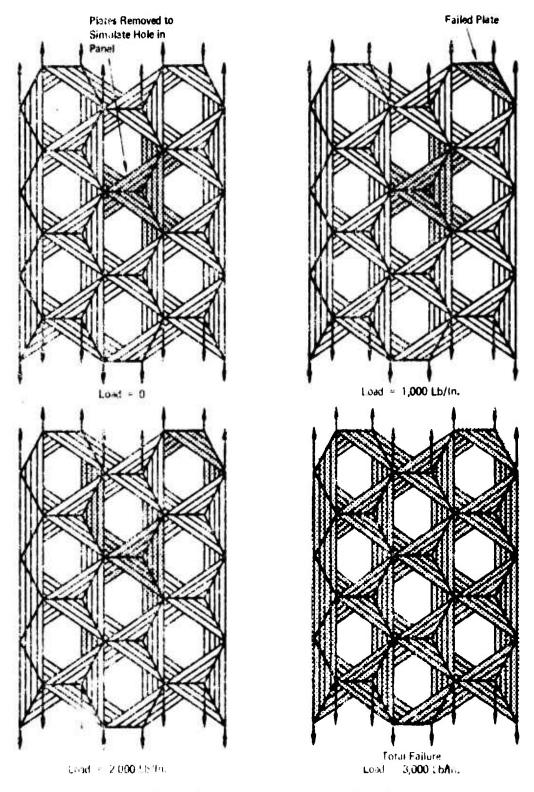


Figure 23. Example of Nontinear Analysis,

BUCKLING ANALYSIS

A buckling analysis is performed on the plate elements in each leg making up the Tetra-core panels in the subroutine BUKL. No equation was available to calculate the compression buckling load of an orthotropic triangular plate with one edge free, loaded in compression. Therefore, it was decided to use the compression buckling equation (developed by Lackman and Ault) for a rectangular orthotropic plate with one edge free and three edges simply supported. The length of the free edge on the triangle was used as the length of the rectangle, with the tetrahedron height used as a rectangle width. This simplification is expected to result in conservative values for the buckling load, due to the greater restraint imposed on the panel by the sides of the triangle compared with the sides of a rectangle. The equation used to calculate the buckling load is

$$\beta \left[\beta^{2} + \left(\frac{\pi b}{a}\right)^{2} \lambda\right] \left[\alpha^{2} - \left(\frac{\pi b}{a}\right)^{2} \mu_{xy}\right] TANH\alpha = \alpha \left[\alpha^{2} - \left(\frac{\pi b}{a}\right)^{2} \lambda\right]$$

$$\left[\beta^{2} + \left(\frac{\pi b}{a}\right)^{2} \mu_{xy}\right] TAN\beta,$$
where
$$\alpha = \left(\frac{\pi b}{a}\right)^{1/2} \left[\left(\frac{D_{3}}{D_{2}} \frac{\pi b}{a}\right)^{2} - \frac{D_{1}}{D_{2}} \left(\frac{\pi b}{a}\right)^{2} + \frac{N_{x}b^{2}}{D_{2}}\right]^{1/2} + \frac{D_{3}}{D_{2}} \left(\frac{\pi b}{a}\right)$$

$$\beta = \left(\frac{\pi b}{a}\right)^{1/2} \left[\left(\frac{D_{3}}{D_{2}} \frac{\pi b}{a}\right)^{2} - \frac{D_{1}}{D_{2}} \left(\frac{\pi b}{a}\right)^{2} + \frac{N_{x}b^{2}}{D_{2}}\right]^{1/2} - \frac{D_{3}}{D_{2}} \left(\frac{\pi b}{a}\right)$$

$$D_{1} = \frac{E_{x}t^{3}}{12 \left(1 - \mu_{xy} \mu_{yx}\right)}$$

$$D_{2} = \frac{E_{y}t^{3}}{12 \left(1 - \mu_{xy} \mu_{yx}\right)}$$

$$D_{3} = \frac{2 G_{xy}t^{3}}{12 \left(1 - \mu_{xy} \mu_{yx}\right)} + \frac{E_{x}\mu_{yx}t^{3}}{12 \left(1 - \mu_{xy} \mu_{yx}\right)}$$

 $\lambda = 4 G_{XY} (1 - \mu_{XY} \mu_{YX}) / E_{Y} + \mu_{XY}$

a = tetrahedron side length

b = tetrahedron height

t = leg thickness

An iteration method is used to find the lowest value of $N_{\rm X}$ which will satisfy the equality. This calculation is made for the Vertical, Skew A, and Skew B legs. If face sheets are applied to the Tetra-core panel, then all edges of the triangle will be simply supported and the equation of a simply supported rectangle is used.

$$N_{x} = 2(\frac{\pi}{b})^{2} [(D_{1}D_{2})^{1/2} + D_{3}]$$

This equation is used for vertical, Skew A and B legs, and upper and lower face sheets when face sheets are applied. If a more accurate buckling equation becomes available, it can easily be inserted into the buckling subroutine.

MARGIN OF SAFETY

The margin of safety is calculated for each plate element using the von Mises-Hill-Tsai equation⁸.

MS = 1/
$$\frac{\sigma_1}{F_x}^2 - \frac{F_y}{F_x} \frac{\sigma_1}{F_x} \frac{\sigma_2}{F_v} + \frac{\sigma_2}{F_y}^2 + \frac{\tau_{12}}{F_{xy}}^2 -1$$

 $\sigma_1,~\sigma_2,~{\rm and}~\tau_{12}$ are the stresses as in the local plate element coordinate system, with σ_1 always in the fiber direction. $F_X,~F_Y,~{\rm and}~F_{XY}$ are the allowable stresses, with F_X being the allowable stress in the fiber direction, F_Y the transverse allowable, and F_{XY} the shear allowable. Tension or compression allowable stresses (as input on card type 10) are used, depending on the sign of the stress. A margin of safety is calculated for the four locations in the plate for which stresses have been calculated, with the lowest margin being printed out for that plate. The margin of safety in buckling is also calculated, using the buckling stress calculated for the leg in which this plate is found.

$$MS_{B} = \frac{F_{C}}{\sigma_{1}} - 1$$

The buckling margin is calculated only if one of the σ_1 's is negative, indicating a compression load in the plate. The lowest margin of safety found in each leg for that load case is printed after printing of the plate element stresses.

CALCULATION OF ULTIMATE LOADS

After the margin of safety in each plate has been determined, the minimum margin of safety found in any plate is used to extrapolate the given applied load to determine the load at which the model would have failed.

$$X_{allow} = X_{load}/(1. - Y_{MS})^{.5}$$
 $Y_{allow} = Y_{load}/(1. - Y_{MS})^{.5}$
 $XY_{allow} = XY_{load}/(1. - Y_{MS})^{.5}$
 $XM_{allow} = X_{mom}/(1. - Y_{MS})^{.5}$
 $YM_{allow} = Y_{mom}/(1. - Y_{MS})^{.5}$
 $XYM_{allow} = XY_{mom}/(1. - Y_{MS})^{.5}$
 $XYM_{allow} = XY_{mom}/(1. - Y_{MS})^{.5}$
 $XQ_{allow} = XQ_{SHR}/(1. - Y_{MS})^{.5}$
 $XQ_{allow} = XQ_{SHR}/(1. - Y_{MS})^{.5}$

where YMS is the minimum margin of safety found in any plate. Xload, Yload, etc., are the input loads from card types 12 and 13. The power of 0.5 may have to be changed to agree with tests when they become available.

CALCULATION OF STIFFNESSES

An equivalent modulus in the x direction is calculated whenever a uniform load is applied in that direction. This is done by taking two x deflections on opposite ends of the panel and adding them to determine the total change in length of the panel (ΔX). This ΔX is divided by the original panel length to get an equivalent strain $^{\varepsilon}_{X_{\hbox{\footnotesize{EQ}}}}$. The uniform load on the x face of the panel, divided by the height of the face, gives an equivalent stress $\sigma_{X_{\hbox{\footnotesize{EQ}}}}$. The equivalent modulus in the x direction is then

$$\mathbf{E}_{\mathbf{x}_{\mathrm{EQ}}} = \frac{\mathbf{\sigma}_{\mathbf{x}_{\mathrm{EQ}}}}{\mathbf{\varepsilon}_{\mathbf{x}_{\mathrm{EO}}}}$$

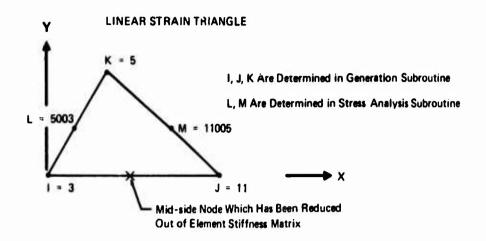
An equivalent modulus in the y direction is calculated in the same way whenever a uniform load is applied to the y face.

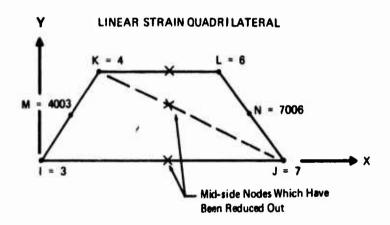
OUTPUT DESCRIPTION

Primary output of the Tetra-core analysis program consists of the finite-element model generated by the data generator overlay and the deflections and stresses calculated by the stress analysis overlay. If the optimization option is used, additional printout is generated during each step of optimization.

The printed output from a standard stress analysis run starts with the input data (number of load cases, size of tetrahedron, leg thicknesses, etc.); the printout of calculated data starts with the number of node points and plate elements generated by the data generator. This node point number does not include the nodes at the midpoint of the triangle or quadrilateral which are added in the stress analysis overlay. The number of load cases is also printed. If a cylinder is generated, the radius to the outer surface is printed. If an airfoil is generated, the chord length is printed. The X, Y, Z coordinates of the generated nodal points are printed. Nodal loads are printed for each load case along with the original node number and the new node numbers that include mid-side nodes added by each linear strain triangle and quadrilateral. Each linear strain element used in the model adds a new node point at the midpoint of two sides of the element where it is attached to another element, which must be accounted for by increasing the size of the load vector and the stiffness matrix. The new node numbers are generated from the node numbers of the vertices on each side of the new node (Figure 24). The new node number is generated by multiplying the larger of the two nodes by 1,000 and adding to the smaller, resulting in a unique new node For example, if a new node is to be generated between nodes 5 and 3, as shown in the figure, the resulting number would be 5003. This node is then added into the stiffness matrix and load vector behind the location of the larger of the two vertex nodes. The added mid-side nodes for a typical case are shown in Figure 26.

Plate properties are printed. The node numbering sequence for each plate in the original node point order and as renumbered is printed. Nodes listed under the headings I, J, K, and L give the original plate connectivity. Nodes listed under LP, LQ, LR, LS, LT, and LU give the connectivity using the new nodal numbering sequence. LS and LT represent new midside nodes for the triangle. Plate moduli and plate thickness are printed along with the node numberings. EX, EY, and EXY have been divided by the factor $1-\mu_{\rm XY}\mu_{\rm YX}$ for use in the element stiffness generator. EX is the Young's modulus in the direction of nodes I and J, and EY is at an angle of 90 degrees to EX.





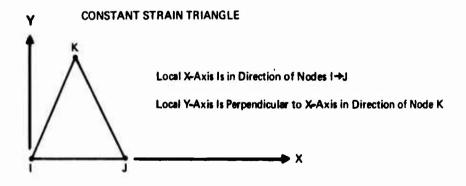


Figure 24. Elements Used in Tetra-Core Program.

Number of Vertex Nodes = 72 Number of Mid-Side Nodes = 96 Total Number of Nodes = 168

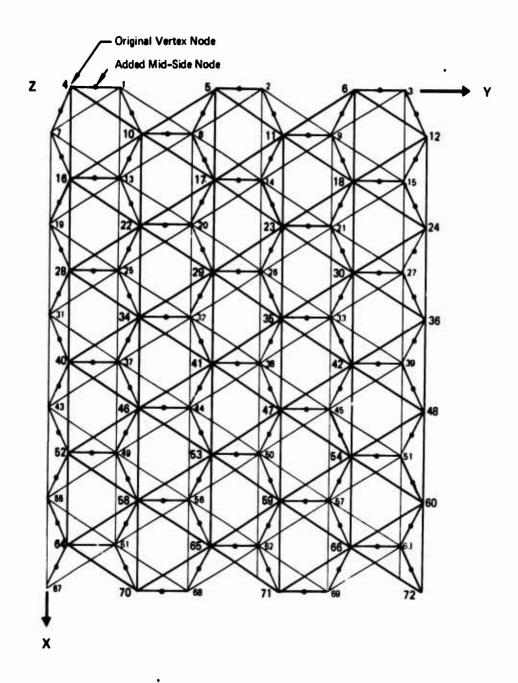


Figure 25. Added Mid-Side Nodes, True Tetra-Core Flat Plate $L_X = 6$, $L_Y = 6$.

The 1/2 bandwidth, in nodes, required to store the stiffness matrix is printed after the elements have been renumbered and printed. The storage space required for the overall stiffness matrix can be calculated by multiplying the number of nodes times 3 times the 1/2 bandwidth times 3. If this number added to the amount of space required to store the loads vectors is greater than the available disc space, then the problem will have to be reduced in size.

Nodal deflections and plate stresses are printed for each load case separately. Deflections are printed for each node with the node points numbered in the original sequence (1, 2, 3, 3001, 4, 4002, etc.). Stresses are printed for four locations in the linear strain element used in Vertical and Skew A and Skew B legs, and for the triangle center for the constant strain triangle used in face sheets. The connectivity of each triangle is printed in the original numbering sequence. For a linear strain triangle, the stresses are printed at node I, node J, node K, and at the center of the triangle, in that order. For a linear strain quadrilateral, which is used only in the truncated Tetra-core model, stresses are printed at nodes I, J, K and L. The margin of safety, calculated using the Hill-Tsai failure criterion, in each element and the leg which that element is in (Vertical leg = leg 1, Skew A = leg 2, Skew B = leg 3, lower face = leq 4, upper face = leg 5) are printed also. The buckling stress of a triangle in each leg is printed.

After all the plate stresses in a load case have been printed, the minimum margin of safety found in each leg is printed. Then the minimum margin of safety is used to extrapolate the original loads to predict a load case with the same ratio between each load that would have a margin of safety of zero. If the model has been loaded in the X or Y direction, the deflections of two node points on opposite ends of the model are used to calculate an equivalent modulus. Then printing of the deflections and stresses for the next load case starts.

If the nonlinear option is specified, a series of load increments will be applied until overall failure occurs. Nodal coordinates and load vectors will be printed out only for the first increment, since the coordinates do not change and the succeeding load vectors will be integer multiples of the first load vector. load intensity (i.e., load in X direction in pounds/inch) is printed at the beginning of each load increment. Plate element moduli are printed at the start of each load increment to allow an inspection of the moduli in each plate to determine the extent of degradation in the panel. As each plate fails in a given direction (longitudinal, transverse, or shear), the modulus in that direction is reduced to near zero. Final failure occurs when all plates have failed. Deflections and stresses are printed out for each load increment.

If the hole option is specified, the moduli of each triangle that connects to a node in the center of the plate, cylinder, or airfoil are set to zero. This node is normally determined by the program but can be input if desired. The identity of the affected node is printed out in either case.

If the optimization option is specified, the coordinates, loads, elements, deflections, and stresses are printed out for the input configuration and for the final optimized configuration Intermediate output during each optimization step consists of the current design configuration and the margin of safety in each leg. When partial derivatives are being calculated, the variable that is being changed is printed out as "IK". Direction cosines of the constant weight surface, direction cosines of the constraint surface of each leg, margin of safety in each leg, and direction cosines of the composite constraint surface are printed at the start of each cycle of sidestepping. Weight of the design is checked at the end of each sidestepping cycle to determine if a new optimum has been reached. If so, the design variables are printed and optimization continues. When the input number of sidestep cycles "NSTP" have been completed, the optimization process is stopped and the deflections and stresses for the lowest weight design that was found are printed.

SAMPLE PROBLEMS

Examples of input for several types of analyses will be given for reference. For a nonlinear analysis, the set of cards shown in Figure 26 will be required. Three sets of cards of Types 8 and 9 are required to input stress-strain curves for each leg. The input cards required for an airfoil analysis are shown in Figure 27. An example of the input for an optimization run is shown in Figure 28.

The output from an optimization run is shown in Appendix II.

ì	Carlo I Abe 3			- Card Type 5. 6												Card Type 8 9	, , , , , , , , , , , , , , , ,												٠ <u>٢</u>				– – Card Type 12	Card Type 13 (Blank)
		1	1	1		0.0	0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	1				1	
						37500.	0.0	47500.	0.0	0.0	.113	,214		37500.	0.0	47500.	0.0	0.0	.113	.214		37500.	0.0	47500.	0.0	0.0	.113	.214						
					.005	31000.	12700.	41500.	12700.	10600.	.120	.227	.005	31000.	12700.	41500.	12700.	10600.	.120	.227	• 005	31000.	12700.	41500.	12700.	10600.	.120	.227	11000.	11000.	11000.			
	25	.25	.25		200:	23000.	9700.	33000.	9700.	9000	.128	.240	.002	23000.	9700.	33000.	9700.	.0006	.128	.240	- 005	23000.	.0076	33000.	9700.	9000	.128	.240	13600.	13600.	13603.			
ESSES	93000	930000	930000	.1	.0025	16000.	6600.	23400.	6600.	7000.	.136	. 252	. 0025	16000.	6600.	23480.	6600.	2000	.136	• 252	• 0025	16000.	.0099	23400.	.0099	7000.	. 136	.252	47400.	47400.	47400.			
RE PLATE STRE		700000	0		0	8500.	3300.	12000.	3300.	****	- 142	• 265	- 005	0050	3300.	12000.	3300.	.0077	.142	. 265	- 005	8500.	300	12000.	3300.	4400.	.142	• 265	13600.	13600.	13600.			
TETRA-CO	£000000	+ 000000	+ 000000	*	• 0025	0.0	0.		0.0		. 15	. 280	_	0.0	0.0	-	9.9	0.0	. 15	. 280	• 0025	••	0.0	0.0		6.0	. 15	. 250	3 8000.	3 6000.	38000.	4 600	•	

Figure 26. Input Data for Nonlinear Analysis.

																				/ !	> Card Type 12, 13	1026	7.000						44							
Card Type 1	Card Type 3		>Card Type 4		Card Type 5, 6	(= = -		-	-			Card Type 7		-				ť	11000.	1																
9		• 25	• 25	. 25	.1				1 1						1			- C 00 C 1	13600		1000															
3 3 0		930303.		936006	.1 .1												2000	47676	47430.																	
	4140	1703333.	1703305.	1700000	1.0	0.0				00.0		C • 5 J	0.63	0.13	6.43	6.93	1.03	13690	13600.																	
3 9 3	TETPA-CO	4000000°	400000°	.000000	3	0.0	40.0	010	4	21.0	0.29	0.25	62.0	0.15	0.13	6.05	9.0	180000	163000.	1000																

Figure 27. Input Data for Airfoil Analysis.

					. 25 > Card Type 4	Ype 5 Ype 6 (Blank)	110001	1100C -> Card Type 10	Card Type 11	Card Type 12	Card Type 13 (Blank)				
	> Card Type 2			d Type 3	50000025 Card Type 4	30 030-Card	3033.	3633.	,						
_	S.			RESSES -Car	7.0000 7.00000 7.00000	. 033 . 033	110000.	113586		133.					
				PLA 1E	200000	.75 .50	13cou.	13500.	.1						
10 A	12	222	12	TE IRA-CORE		\$	110036.	.00011	.1	1900.					

Figure 28. Input Data for Optimization Run.

COMPARISON WITH UNIVERSITY OF KANSAS TESTS

Computer runs have been made to compare to several of the tests run at the University of Kansas. The results of this comparison are not very meaningful, however, because the basic properties of the material making up the legs were not specified in the test document. Information in the test document was limited to side length and tetrahedron height and the fiber used, which was S glass. The program requires elastic moduli and thickness for each leg. It was assumed that the material had the following moduli and thickness:

 $E_{X} = 4,000,000 \text{ psi}$ $E_{Y} = 500,000 \text{ psi}$ $G_{XY} = 500,000 \text{ psi}$ $\mu_{XY} = 0.25$ t = 0.030 in.

Using these as input with a side length of 0.6123 inch and a height of 0.4 inch (comparable to specimen 1 in Reference 9) produced an Ex of 281,000 psi and an Ey of 284,000 psi. The predicted initial failure load, using a linear stress analysis, was 789 pounds. The cause of this failure was a high transverse tension stress in some Skew A and B plates which would not have resulted in final failure. If the criterion of failure had been fiber stress, the predicted ultimate load would have been 3,300 pounds for a fiber stress of 600,000 psi. This compares to an Ex of 363,053 psi, an Ey of 303,198 psi, and an ultimate load of 5,800 pounds from the test. However, a comparison between the analysis and test would be risky, since material properties of the specimens used for the test were guessed at. The low predicted load of 789 pounds was caused by failure of the Skew A and B legs in the transverse direction.

An analysis run with a configuration comparable to that of Specimen 3 (HT = 0.5, side = 0.75) was run, resulting in an Ex of 293,400 psi that compares to the test result of Ex = 288,150 psi. This appears to be good agreement, assuming that the input moduli and thickness were close.

MODEL CONSTRAINTS

The Tetra-core analysis program is dimensioned for 1100 plate elements, 300 vertex nodes, and 600 total nodes. A problem of this size cannot be run on the IBM 360/44 due to a lack of disc space. However, the program can be converted to a larger 360, such as a 360/65, very easily, should one become available. This can be done by removing the call load cards which are used to load a new overlay before referring to it. These cards are not necessary on the operating system used by the larger IBM machines. Also, job control cards must be changed. With a larger machine it will be possible to increase the size of problem that may be run to 600, or greater vertex nodes. This can be done by increasing the X, Y, Z and UX, UY, UZ dimensions in the generation and stress analysis subroutines. The dimensions of variables A and DIAG in subroutine STIFF must be increased. In subroutine SOLPAC, variables DIAG, AI, AJ, B, and X must be increased in size. In subroutine DEFL, variables IDO and B must be increased in size.

In converting to a CDC 6600 machine, it will be necessary to put in OVERLAY and CALL OVERLAY cards for each overlay. Direct access read/write cards used for the IBM 360 will not work on the CDC 6600. Direct-access read/write commands will have to be rewritten for the particular CDC 6600 on which the program is to be run, since they vary from installation to installation.

The truncated Tetra-core model can generate the full range of Tetra-core geometries, from a true Tetra-core to a truncated Tetra-core with vertical sides. However, both the true Tetra-core and the vertical-sided Tetra-core models will have several nodes which are connected to each other by plates generated for the same X, Y, Z coordinates. This will cause a linear dependence in the stiffness matrix, since these nodes will essentially have the same stiffnesses; that is, their rows and columns will be identical. This will result in the matrix being singular, and no solution can be found. This problem will also result when the connected nodes are within a small distance of each other.

The program does not compute nodal reaction force, since this would require more disc storage space than is available on the IBM 360/44 at Fort Eustis. If the program is converted to a larger machine, reactions can be calculated by storing a copy of the merged stiffness matrix and then multiplying it by the calculated deflections: $P = K\Delta$.

COMPUTER PROGRAM

Input Formats for Tetra-Core Analysis Program

CARD TYPE	VARIABLE	FIELD	COMMENT
	VARCIABED	11000	COPERATI
1	NLOD	I (1-4)	Number of load cases to be run. If "IOPT" = 1 or "NLIN" = 1 then NLOD must = 1.
	IOPT	I (5 -8)	If "IOPT" = 0, no optimization will be done. If "IOPT" = 1, the Tetra- core element will be optimized for the given load case.
	NLIN	I (9-12)	"NLIN" = 0, standard linear stress analysis will be run. If "NLIN" = 1, a nonlinear analysis will be run using the input load case as the load increment and "NSTP" as the number of iterations.
	ITYP	I (13-16)	<pre>If "ITYP" = 1, flat plate analysis If "ITYP" = 2, cylinder analysis If "ITYP" = 3, airfoil analysis</pre>
	IFACE	I (17-20)	<pre>If "IFACE" = 0, no face sheets will be added to the Tetra-core. If "IFACE" = 1, face sheets will be added.</pre>
	IHOLE	I (21-24)	If "IHOLE" = 0, a standard analysis will be run. If "IHOLE" = 1, all plates connected to a grid point in the center of the panel will be defined with zero stiffnesses, giving the effect of a hole in the panel. If "IHOLE" > 1, all plates connected to grid point "IHOLE" will have zero stiffness, allowing a hole to be inserted where desired.
	NSTP	I (25-28)	"NSTP" determines the number of iterations to be made in a nonlinear analysis, or the number of optimization cycles to be made in an optimization run. The default option for "NSTP" is 3.

CARD	VARIABLE	מוקדם	COMMENT
TYPE	VARIABLE	FIELD	COMMENT
	ITET	I (29-32)	<pre>If "ITET" = 0, standard tetrahedrons will be generated. If "ITET" = 1, truncated tetrahedrons will be generated.</pre>
	ITEST	I (33-36)	If "ITEST" = 1, the element and merged stiffness matrices will be printed out. If "ITEST" = 0, no printout of matrices.
	ILOD	I (37-40)	If "ILOD" = 0, the program will automatically generate nodal loads and fixities. If "ILOD" > 0, nodal loads and/or fixities are input on Card Types 14 and 15, as shown below.
	ILOD	Comment	
	1		s are input on Card Type 14. ties are automatically generated.
	2		s are input on Card Type 14. ties are input on Card Type 15.
	3		s are automatically generated. ties are input on Card Type 15.

CARD TYPE	VARIABLE	F	[ELD	COMMENT
				William
2	ISA	Ι	(1-4)	Card Type 2 is input only if "OPT" = 1. "ISA", "ISB", "ISC" are used to
	ISB	I	(1-5)	set variables equal to each other during optimization. Il cards of type 2 are input.
	ISC	I	(9-12)	During optimization the program will normally change each variable independently. ISA, ISB, ISC can be used to keep several variables, such as leg thicknesses, the same during optimization. ISA can also be used to hold variables constant during optimization by inputting ISA = 12 for that variable. One Card Type 2 is input for each variable, resulting in eleven cards of type 2. The first Card Type 2 is used to control the side length, the second controls height, the third controls vertical leg thickness, etc., as shown below. If ISA = 2 on the first card type 2 and ISA = 1 on the second card type 2 then the first and second variables (side length and height) will be kept the same during optimization. The program changes only the leg thicknesses during the first descent cycle when going from the input design to the initial design on the constraint curve. Therefore, at least one leg thickness must be allowed to vary during optimization or the program will not be able to find an initial feasible design point since it will not have anything to vary.

CARD TYPE 2 NUMBER	VARIABLE CONTROLLED	ISA	ISB	ISC
1	Side Length	12		
2	Height	12		
3	Vertical Leg Thickness	4	5	
4	Skew A Leg Thickness	3	5	

CARD TYPE 2 NUMBER	VARIABLE CONTROLLED	ISA	ISB	ISC
5	Skew B Leg Thickness	3	4	
6	Upper Face Sheet Thickness	12		
7	Lower Face Sheet Thickness	12		
8	THETA	12		
9	Yloff	12		
10	Y2OFF	12		
11	THT	12		

TYPE	VARIABLE	FIELD	COMMENT
3	Title	A(1-48)	Title Card

CARD			
TYPE	VARIABLE	FIELD	COMMENT
4	EX	F(1-12)	Elastic modulus in longitudinal direction of leg, psi.
	EY	F (13-24)	Elastic modulus in transverse direction of leg, psi.
	GXY	F(25-36)	Shear modulus of leg,psi.
	μ_{XY}	F(37-48)	Poisson's Ratio
			One card type 4 must be input for each leg (Vertical Skew A, Skew B) and for the upper and lower faces if "IFACE" = 1.

CARD			
TYPE	VARIABLE	FIELD	COMMENT
5	LX	I(1-4)	Number of legs in X face of Tetra-core "LY" times the length of a tetrahedron times 0.866 gives the panel width.
	LY	I (5-8)	Number of legs in Y face of Tetra-core "LY" times the length of a tetrahedron side gives the panel length.
	SIDE	F(13-18)	Length of a tetrahedron side, in.
	HT	F(19-24)	Height of Tetra-Core Panel, in.
	THK (1)	F(25-30)	Thickness of Vertical Legs, in.
	THK (2)	F(31-36)	Thickness of Skew A Legs, in.
	THK (3)	F(37-42)	Thickness of Skew B Legs, in.
	THK (4)	F(43-48)	Thickness of Lower Face Sheet, in.
	THK (5)	F(49-54)	Thickness of Upper Face Sheet, in.
			"THK(4)" and "THK(5)" are input only if "IFACE" = 1 on card type 1.

CARD TYPE	VARIABLE	FIELD	COMMENT
6	ТНЕТА	F(1-12)	The variables input on this card control the variation of of the Tetra-core element from
	Yloff	F(13-24)	a true tetrahedron. See Figure 4 for a description.
	Y20FF	F(25-36)	
	THT	F(37-48)	

CARD TYPE	VARIABLE	FIELD	COMMENT
7	TC	F(1-12)	Thickness/Chord Ratio
	хс	F(13-24)	X/Chord Ratio

Card Type 6 is input only if "ITYP" = 3 on card 1, that is, only if an airfoil shape is to be generated. Eleven cards of type 6 must be input, each with a thickness/chord for the specified X/Chord. The X/Chord at the nose of the airfoil is input first, with X/Chord for the tail of the airfoil last. The intermediate X/Chord cards must be in sequence from nose to tail.

CARD			
TYPE	VARIABLE	FIELD	COMMENT
8	STT1	F(1-12)	Longitudinal tension strain increment, in./in.
	STT2	F(13-24)	Transverse tension strain increment, in./in.
	STC1	F(25-36)	Mongitudinal compression strain increment, in./in.
	STC2	F(37-48)	Transverse compression strain increment, in./in.
	STSS	F(49-60)	Shear strain increment, in./in.
			"STT1", "STT2", "STC1", "STC2", "STSS" are the strain increments used in inputting tension and compression stress-strain curves on cards of Type 9. The stress- strain curves are broken down into increments of "STT1," "STT2," etc., for inputting on card 9.

CARD TYPE	VARIABLE	FIELD	COMMENT
9	SIGT(I) I = 1,7	F(13-25)	Stress-strain curves of the material used in each leg and each face sheet are input if a nonlinear analysis is performed ("NLIN" = 1, card 1). Longitudinal tension stresses are input for strains of 0, STT1, 2STT1, 3STT1,6STT1, for example. The first card following card 8 must contain the longitudinal tension stresses; the second card contains the transverse tension stresses, etc. The complete sequence is shown below.

CARD TYPE	VARIABLE	CONTENTS
8		Strain increments - "STT1," "STT2," "STC1," "STC2," "STSS.", in./in.
9	SIGT1	Longitudinal tension stresses in increments of strain "STT1.", psi.
9	SIGT2	Transverse tension stresses in incrents of strain "STT2.", psi.
9	SIGC1	Longitudinal compression stresses in increments of strain "STCl.", psi.
9	SIGC2	Transverse compression stresses in increments of strain "STC2.", psi.
9	PRXT	Poisson's ratio μXY in tension in increments of "STT1."
9	PRXC	Poisson's ratio µXY in compression increments of "STC1."
		One set of Card 8 + six card 9's must be input for each leg of the Tetra-core. If "IFACE" = 0 on card 1, then three sets must be input-Vertical legs, Skew A legs, and Skew B legs. If "IFACE" = 1, then five sets must be input- three sets for the legs, plus one set for the lower face sheet, and one set for the upper face sheet.

CARD TYPE	VARIABLE	FIELD	COMMENT
10	FXT	F(1-12)	Longitudinal tension allowable stress, psi.
	FYT	F(13-24)	Transverse tension allowable stress, psi.
	FXC	F(25-36)	Longitudinal compression allowable stresses, psi.
	FYC	F(37-48)	Transverse compression allowable stress, psi.
	SSS	F(49-60)	Shear Allowable Stress, psi.
			One card type 9 must be input for each leg. If "IFACE" = 1, five card 9's are input, one each for the Vertical legs, Skew A legs, Skew B legs, lower face sheet, upper face sheet. If "IFACE" = 0 only the first three are input.

CARD TYPE	VARIABLE	FIELD	COMMENT
11	RHO (1)	F(1-12)	Card 10 is input only if "IOPT" = 1 on card 1. Density of material used in vertical legs, lb/in. ³ .
	RHO (2)	F(13-24)	Density of material used in Skew A legs, lb/in.3.
	RHO (3)	F(25-36)	Density of material used in Skew B legs, lb/in.3
	RHO (4)	F(37-48)	Density of material used in lower face sheet, lb/in.
	RHO (5)	F(49-60)	Density of material used in upper face sheet, lb/in.3.
			The density is used to calculate the weight of the Tetra-core for use in optimization.

CARD			
TYPE	VARIABLE	FIELD	COMMENT
12	XLOAD	F(1-12)	Load on XFACE of flat plate Tetra-core, lb/in; or total end load on cylinder or airfoil, lb.
	YLOAD	F(13-24)	Load on YFACE of flat plate Tetra-core, lb/in.
	XYLOD	F(25-36)	Shear load on flat plate Tetra- core, lb/in.
	хмом	F (37-48)	Moment on XFACE of flat plate Tetra-core in lb/in.; or total moment on cylinder or airfoil, in-lb.
	YMOM	F(49-60)	Moment on YFACE of flat plate Tetra-core inlb/in.
	MOMYX	F(61-72)	Twisting moment on flat or plate Tetra-core inlb/in.
	TORQ	F(73-80)	Torque on cylinder or airfoil of Tetra-core, inlb/in.

CARD TYPE	VARIABLE	FIELD	COMMENT
13	XQSHR	F(1-12)	Shear load on XFACE of flat plate Tetra-core, lb/in.
	YQSHR	F(13-24)	Shear load on YFACE of flat plate Tetra-core, lb/in.
			There is an upper limit of 10 on the number of load cases that may be run.
			If a nonlinear analysis is to be made (NLIN" = 1) or an optimization is to be run ("IOPT" = 1), only one load case is allowed.

CARD TYPE	VARIABLE	FIELD	COMMENT
14	J	I (1 - 4)	Card type 14 is input only if "ILOD" = 1 or 2 on card 1. Node which loads input on this card will be applied to.
	UX (J)	F (12-24)	Load in X direction on node J
	UY (J)	F (25-36)	Load in Y direction on node J
	UZ (J)	F (37-48)	Load in Z direction on node J
		being load	e 14 is input for each node ed. Input of Card Type 14 is by a card with J = 300.

CARD			
TYPE	VARIABLE	FIELD	COMMENT
15			Card Type 15 is input only if "ILOD" = 2 or 3 on Card 1.
	J	I (1 - 4)	Node for which fixity is read in.
	Code (J)	F (13-24)	Fixity of node J. Convention shown below is used.
	Code (J)	Freedoms Fi	ixed
	0.0	None	
	1.0	x	
	2.0	Y	
	3.0	Z	
	4.0	Х, У	
	5.0	X, Z	
	6.0	Y, Z	
	7.0	X. Y. Z	

Card Formats

Input to the Tetra-core analysis program must be in the form of data cards punched according to fixed formats.

Integer Fields: I (1-4), I (26-30), etc.

An integer must be right adjusted in the field. Unneeded field space may be left blank. Blanks are interpreted as zeros in the corresponding column. Decimal points are not used.

Decimal Fields: F(1-12), F(37-48), etc.

A decimal number, punched with a decimal point, may be located anywhere within the field. Blank fields are interpreted as zeros.

Alphanumeric Fields: A(1-27), A(21-30), etc.

Alphanumeric characters may be anywhere in the field. Legal characters for this field include all FORTRAN characters.

LIST OF SUBROUTINES

OVERLAY TETRA 1 - ROOT OVERLAY

- MAIN Main program. Calls subroutines GEN, STRSS, SOLPAC, DEFL.
- OUTIN Used to read and write stiffness matrix and load vectors on file 4. Called by STIFF, SOLPAC, DEFL.

OVERLAY TETRA 2

- GEN Subroutine to read and print input data. Calls subroutines NODGEN, PLATEN, LODGEN, NODGAN, PLATGA, LODGAN, OPTM, NONLIN. Called by MAIN.
- CURV Curve fitting subroutine, used in generation of airfoil shape. Called by NODGEN, NODGAN.
- OPTM Optimization subroutine. Determines direction of travel to obtain minimum weight design. Calls WEIGHT, DAR. Called by GEN.
- WEIGHT Subroutine to calculate weight of Tetra-core model being optimized. Calls NODGEN, NODGAN. Called by OPTM.
- DAR Subroutine to calculate direction cosines of gradient given partial derivatives. Called by OPTM.
- NONLIN Subroutine to increment loads before each step of nonlinear analysis. Called by GEN.

OVERLAY TETRA 3

NODGEN - Subroutine to generate node points for true Tetra-core model. Calls CURV.

OVERLAY TETRA 4

PLATGN - Subroutine to generate plates for true Tetracore model. Called by GEN.

OVERLAY TETRA 5

LODGEN - Subroutine to generate nodal loads and nodal fixities for true Tetra-core model. Called by GEN.

OVERLAY TETRA 6

NODGAN - Subroutine to generate node points for truncated Tetra-core model. Called by GEN, WEIGHT.

OVERLAY TETRA 7

PLATGA - Subroutine to generate plates for truncated Tetra-core model. Called by GEN.

OVERLAY TETRA 8

LODGAN - Subroutine to generate nodal loads and nodal fixities for truncated Tetra-core model. Called by GEN.

OVERLAY TETRA 9

STRSS - Overlay to perform finite-element stress analysis on model generated by previous overlays. Called by MAIN. Calls STRESS, STIFF.

OVERLAY TETRA 10

STRESS - Subroutine to renumber Tetra-core model to account for midside nodes. Called by STRSS.

NID - Function to generate new number for midpoint node.

OVERLAY TETRA 11

- STIFF Subroutine to merge element stiffness matrices into overall stiffness matrix. Called by STRSS. Calls TRPRD, TRIM6, OUTIN.
- TRPRD Subroutine to perform matrix multiplication $[A]^T[B][A]$. Called by TRIM6.
- TRIM6 Subroutine to generate linear strain triangle and quadrilateral and constant strain triangular stiffness and stress matrices. Called by STIFF. Calls TRPRD.

OVERLAY TETRA 12

SOLPAC - Subroutine for solution of stiffness matrix.

Solves Eqn P = KA using Choleski triangularization and back substitution. Called by MAIN.

OVERLAY TETRA 13

- DEFL Subroutine for printing of deflections, calculation of plate element stresses. Called by MAIN. Calls MARGIN, MODUL, BUKL, PLST.
- MARGIN Subroutine to calculate margin of safety in each plate element using von Mises-Tsai failure criterion. Called by DEFL.
- MODUL Subroutine to calculate plate secant moduli based on strains within the plate. Called by DEFL.
- BUKL Subroutine to calculate buckling stress for typical plate from each leg. Called by DEFL.
- PLST Function to calculate stress given the strain in a plate.

DESCRIPTION OF VARIABLES USED IN COMMON

COMMON/COMN/

NUMNP - Number of nodal points in model

NUMEL - Number of plate elements in model

NLOD - Number of load cases

NLIN - Used to control noninear analysis

MBAND - 1/2 bandwidth of stiffness matrix, in freedoms

NBAND - 1/2 bandwidth of stiffness matrix, in nodes

COMMON/MATL/

E - Moduli of elasticity for each leg

FXT, FYT, FXC, FYC, SSS - Allowable stresses of material in each leg

CMS - Margin of safety in each leg

COMMON/LODA/

XLOAD, YLOAD, XYLOD, XMOM, YMOM, XYMOM, TORQ, XCHR, YQSHR - input applied loads

COMMON/ONE/

THK - Thickness of each leg

THETA, Y10FF, Y20FF - Parameters to control variation from theoretical equalateral tetrahedron (see Figure 4)

THT - Theoretical height at which tetrahedron sides would meet (see Figure 4)

DUM - Not used

TC, XC - Thickness/chord and X/chord ratios used to generate airfoil model

HED - Title

NX, NY - Number of legs on X and Y face. Called LX and LY on input sheets (see Figure 5)

IOPT - Used to control optimization

IFACE - Control of face sheet generation

IHOLE - Used to control if hole will be placed in model
 or not

ITET - Used to control type of tetrahedron generated-true or truncated

ICNT - Counter for number of steps that have been run
in nonlinear or optimization analysis

NNEW - Used to determine which leg a given plate is in

COMMON/OPTA/

NSTP - Limiting number of steps for nonlinear analysis. limiting number of cycles for optimization

- IDS Controls steepest descent mode in optimization
- IDRV Controls calculation of derivatives in optimization
- ITOT Counter for number of cycles in optimization
- ICYC Counter for number of sidesteps in optimization
- LAST Not used
- IK Used to identify variable being incremented when calculating derivatives in optimization
- WTL Base weight before start of calculation of derivatives in optimization
- XMSL Margin of safety at start of last sidestep
 in optimization
- XMS Present margin of safety
- WTMN Minimum weight found in optimization
- TMLT Step length used in sidestepping optimization
- WT Present weight
- HMAX Value of variables associated with WTMN optimization
- HL Value of variables at start of calculation of derivatives - optimization
- CMSA Value of margins of safety at start of calculation of derivatives optimization
- SLOPE Partial derivatives of effect of variables on margin of safety optimization
- RHO Density of material in each leg
- PWT Gradient of constant weight surface optimization
- PMT Composite constraint gradient in optimization
- ISA, ISB, ISC Used to control variables during optimization

COMMON/MODU/

- SIGT1, SIGT2, SIGC1, SIGC2, SIGSS Stresses defining stress-strain curves used in nonlinear analysis
- PRXT, PRXC Poisson's ratios used in nonlinear analysis
- STT1, STT2, STC2, STSS Strain increments used to define stress-strain curves for nonlinear analysis

COMMON/LODB/

APPLD - XLOAD, YLOAD, etc. for each load case are stored in APPLD

COMMON/RAX/

- NR4 Pointer for next record to be read on File 4
- NR4LOD Gives location of first record in loads vector on File 4
- NR4STF Gives location of first record in stiffness matrix on File 4
- NR4DIA Gives location of first record in diagonal on File 4
- NREC Number of records required to store three rows of stiffness matrix on File 4
- NRECST Number of records in stiffness matrix
- NRECLD Number of records in loads vector
- NR1 Pointer for next record to be read on File 1
- NR2 Pointer for next record to be read on File 2

DESCRIPTION OF VARIABLES USED IN SUBROUTINES

- SUBROUTINES GEN, NODGEN, PLATGN, LODGEN, NODGAN, PLATGA, LODGAN, WEIGHT
 - X, Y, Z Nodal coordinates
 - SLOPA Slope of surface at each node--used for airfoil
 - NA, NB, NC, ND Nodes at vertices of plate element

CODE - Fixity for each node

UX, UY, UZ - Loads on each node

SUBROUTINE - OPTM

PMS - Gradient to each margin of safety surface

H - Vector of variables to be optimized

SUBROUTINE - STRESS, STIFF, TRIM6

X,Y,Z,UX,UY,UZ,CODE - See subroutines NODGEN, PLATGEN, etc.

IDO - Node numbers as renumbered to include mid-side nodes

ID - Node numbers in original order, not including midside nodes

NI - Node number of new mid-side node

IN - Sequence of new mid-side node

ML - Added mid-side nodes in plate element

LM - Original vertex nodes inplate element - in original nodal numbering sequence

MM - Renumbered mid-side and vertex nodes in plate element

DUM1,DLM2,DUM3,DUM4 - Dummy variables used in renumbering of load vector and nodal fixities to include the effect of added midside nodes

S - Element stiffness matrix

ETR, ELGT, EPRM, G - Plate elasticity matrix - ETR = EY/(1- $\mu_{xy}\mu_{yx}$), ELGT = Ex/(1- $\mu_{xy}\mu_{yx}$), EPRM = Ey $\mu_{xy}/(1-\mu_{xy}\mu_{yx})$, G = GXY

T - Plate thickness

ST - Plate stress matrix

IMTL - Indicates leg that plate is in

A - Three rows of merged stiffness matrix

DIAG - Diagonal of merged stiffness matrix

PHYX, PHYY - Used in computing element stiffness matrix

ALAM - Element transformation matrix. Transforms stiffnesses from local to global coordinates

AL, AL2 - Direction cosines of element

QMAT - Elasticity matrix

AST, AK21, DUM, STA, STR, STRA - Dummy matrices used in calculating stiffness and stress matrices

ZETA - Area coordinates of location in triangle for which stresses are calculated

IK - Order in which triangle stiffnesses are added into quadrilateral stiffness matrix

SUBROUTINE SOLPAC

DIAG - Diagonal of stiffness matrix

AI - Three rows of stiffness matrix

AJ - Three columns of stiffness matrix

B - Initial loads vector, deflections after solution is complete

X - Loads vector after reduction

SUBROUTINES DEFL, BUKL, MODUL

P - Displacements for nodes connecting to an element

LM, MM, ST, IDO - See subroutine STRESS

STRESS - Element stresses

FBKL - Buckling stress of an element in each leg

STRAIN - Element strains

B - Nodal deflections

D - Plate stiffnesses

XL - Width of plate

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PA) OPTH (K.Y.Z.SLOPA) NONLIN (ICNT.10PY, LOPA) LOPA)	LEVEL 4	BLOCK / SYMBOL NLOD NEL	BLOCK / SYMBOL FYT	BLOCK / SYMBOL XYLOD XOSHR	SYMBOL SYMBOL THK DUM NY 1TET	SYMEOL SYMEOL 195 10 THLT SLW 15A
	VERSION 3. L	COMMON LOCATION 000004 000018	COMMON LOCATION 000050 000084	COMMON LOCATION 000004 000018	COMMON COCATION COCCE COCCE COCCC	COMMON 0000004 000018 00003C 000090
	S d	SYMBOL NUMEL NBN	SYMBOL FXT CMS	SYMBOL YLOAD TORQ	SYMBOL HT THT 4X IHOLE	SYBOL IPRT LAST MITIN CRSA PRT
200 CONTINUE 200 C	IV MODEL 4	L3CATION 000000 000014	LDC ATION 000000 000000	LOCATION 000000 0000014	LOCATION 000000 000024 000089	LOCATION 000000 000014 000054 000064
0000 0100 0100 0103 0103 0103 0104 0105 0106 0116 0111 01111 01112 0113 0113 0114 0117 0117 0118	FORTRAN IV	SYPBOL NUNNP NUNBLR	SYMBOL E SSS	SYMBGL MLOAD KYMON	SYMBOL SIDE YZOFF MED 1FACE	SYMBOL NSTP 1CYC XMS MI PMT

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L OCA TION 0002 D0 0005 14	LOCATION	LOCATION	LOCATION 00021C		LOCATION	LOCATION 0014F0 001504	LOCATION 001926 001810 001840 002348 0024A0	002 7 BA			
SYMBOL S1355 STCL	SYMBOL	SYMBOL	SYMEOL	PAGE 0005	× ×	SYMBOL PLATGN OPIN	LABEL 6120 5010 6035 6035 160	2	i		
LOCATION 00021C 000500	LOCATION	LOCATION	LOCATFON 000218		L DCATION 001330	LOCATION 0014EC 001400	LCATION 0018F8 0018F8 00106C 0022CC 00248A 00248A				
SIZE 000550 Symbol Si3C2 St7Z	SIZE DODIES SYMBOL	SIZE 000004 SYMBOL	SYMBOL		SYMBOL	SYMBOL NOGEN LODGAN	LABEL 6110 300 6035 5035 155 230				
MODU / HAP COCATION ODOLGS	LOOB / MAP !	DUMP / MAP S LOCATION	LOCATION 000214	DATE 71321	LOCATION 000880	LED LOCATION 0014E8 0014FC	LGCATION 001814 001848 001558 002254 002572 002574		ŀ		
SYMBOL SYMBOL SIGCI SITI	SYMBOL SYMBOL	SYMBOL /	SYMBOL JK	LEVEL 4	NAP SYMBOL Z	SUBPROGRANS CALLED ON SYMBOL I LOAD LOGGN (1.48EL 6010 6015 5040 120 120 150 6080				1)
COMPON LOCATION 0000184 000538 00053C	COMMON LOCATION	COMMON LOCATION	SCALAR LOCATION 050210	VERSION 3.	ARRAY LOCATION 000600	SUBPRO LDCATION 0014E4 0014F8	LOCATION 001760 001760 00127 002248 002248 00255 00255	U	CURV (X,Y,A,B,C)	= 1000000. = 0.0 100 I = 1.11 (X - A(I)) 110.100.100 VTINUE VTINUE	8(J)) / (A(I)—A(J)) (X-A(J)) • C ((B(I)—B(J))•(A(I)—A(J)))
SYMBOL S1612 PRXC S155	TOBHAS	SYMBOL	SYMBOL	\$4 \$	SYMBOL	SYMBOL EXIT PLATGA	LABEL 5020 5020 5025 6020 110 145 5050	MENTS OF	CD.MAP)		C = (B(I)-B(J)) Y = B(J) + (X-A C = ATANZ ((B(I CONTINUE RETURN END
LOCATION 000000 000384 000528	LOCATION 000000	LOCA TI DN 000000	LDCAT 00020	IV MODEL	LOCATION 000 220	LOCATION 0014E0 0014F4 00150B	LOCATION 001632 001954 001846 00244 00246	MEMORY REQUIRE	EXEC FORTRANCE	100 CO	20 CS
SYMBOL SIGTI PRKT STC2	SYMBOL	SYMBOL I TEST	SYNBOL	FORTRAN	SYMBOL	SYMBOL IBCOME NODGAN NONL IN	LABEL 50 6130 5030 5045 6075 200		//CURV E		0011 0012 0013 0014 0015

	LOCATION 0000F0	LOCATION	LOCATION	LOCATI 00														
P AGE 0002	SYMBOL	SYMBOL	SYMBOL	LABEL			PAGE 0001											†
	LOCATION	LOCATION	LOCATION	LOCA 710M				20 20 20 20 20 20 20 20 20 20 20 20 20 2			!		4		1		EPS 6.	
	TC9WAS	SVMBOL	SYMBOL	LABEL				K,NBV,VEL(20) HY,DUM,TC(11) T,ICNT,NMEH(4) 1,555(5),GMS(1)	_				· -				4. 5K. THST	
DATE 71321	L OCATION 0000E8	LOCATION	LED LOCATION	LOCATION 000292			DATE 71321	NE OPTM(X, Y, Z, SLOPA) SOMY / NUMNP, NUMEL, NLOD, NLIN, MBAND, NUMBLK, NBY, VEL(20) ONE / SIDE, HT, THK (S), THETA, Y LOFF, Y 20FF, THT, DUM, TC(11)), HED(112), MX, NY, ROPT, I TYP, I FACE, I HOLE, I FEF, I CMY, NNEMIG, MATL / E(4,5), FXT(5), FYT(5), FXC(5), FYC(5), SSS(5), CMSILO) OPTA / NSFP, PRF, 105, 1084, ROT, RCC, ST, RA, MT, SMSL	MAX(11).H.(11),CMSA(11) O(5), PWT(11),PMT(11)	(300)			WRITE (6, 6005) STEPS ///)	•			150 ICNT.1TOT . 14, 5x, 11HSIDESTEPS #, 14, 5x, 7MSTEPS	
LEVEL 4	SCALAR MAP ON SYMBOL I	Y MAP SYMBOL	SUBPROGRAMS CALLED ON SYMBOL	L MAP LABEL 120			LEVEL 4	194) 161,NLOD.NL I R(5),THETA, 10PT,ITYP,IF (115),FYT(5)	HMAX(11).H	5),H(12)			T . EG. 13 W	. 5 .x.Y.Z.SLOP		- CHS(1)	.NT,170T 14, 5X, 11HS	H(1). I-1.111
VERSION 3.	SCAL LOCATION 0000E4	ARRAY LOCATION 0000F 8	SUBP LOCATION	LOCATION 000212	S 0002E0 BYTES	0	VERSION 3.	E \ 2 \	.wfmw.fplf.df. HMAX(11).kf.(11).CMSA(11).SLOPE(11.5).RHO(5).PWT(11).PWT(11).	ION SLP (11), PHS(11,5), H(12) ION X(300), Y(300), Z(300), SLOPA(300) LENCE (H,SIDE)	5E20.5 } 10F12.5 }	3. 1) N . S	.EG. 1 .AVD. IPRT .EG. 1) 1H1.//.SX.18-40PIIMIZATION		T.	.GT. CMS(11) XMS	RT - 1) 150,140,150 (6,6100) [K,1CYC,1CNT,1TOT (/, 5x, 4HIK #, 14, 5x,	5x, 8HCYCLES # , 14 } (6,6110) MT,XMS,(M(I),I=1,11)
\$	SYMBOL	SYMBOL	SYMBOL	LABEL 110	REMENTS 0002	RITY CODE MAS	44 PS	SUBROUTT OFFON / OMFON / CC 11	SLW	OT MENSION DIMENSION SQUIVALENCE	1PQT = 1 6000 FORMAT (5E20.5 6020 FORMAT (10F12.	ACE	ž~	ICNT = ICNT + IF (ICNT + GT.	XMS = 10.	S.	140 WRITE (6.610)	14.
I IV MODEL	LOCATION 000060	LOCATION 0000F4	L DC A T 1DN 0000F C	L OCA TION 0001 F6	MEMDRY REQUIREMENT	HIGHEST SEVERITY (EXEC FORTRANIBED,	IV MODEL 44		3 8		6000		\$009		1	130	140	
FORTRAN	SYMBOL	SYMBOL	SYMBOL ATAN2	LABEL 100	TOTAL	//OP TIN	FORTRAN 1V	0001 0003 0004 0004	1	0000 0000 \$000	0100 0100	0012	9100	0017	0020	0022	0024 0025 0026	0027

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C CALCULATE DIRICTION COSINES OF CONSTANT WEIGHT SURFACE

220 CALL DAR (SLUPE, PMS, N, M, 1SA)

CALL DAR (SLOPE, PMS, N, M, 1SA)

DO 230 J = 1, N

SLP(J) = 0.

DO 230 K = 1, N

SLP(J) = 0.

DO 230 K = 1, N

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DO 230 K = 1, N

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DO 230 K = 1, N

SLP(J) = 0.

CALCULATE DIRECTION COSINES OF RESULTANT CONSTRAINT SURFACE

CALCULATE DIRECTION COSINES OF RESULTANT CONSTRAINT SURFACE

CALCULATE (6, 6030)

ZAO MRITE (6, 6030)

ANTIFORM CALCULATE OF SAFETY (7, SW, 46H, DIR. COSINES OF COMPOSITE CO

3NSTRAINT SURFACE

JOST COMPOSITE COMPOSITE CO
                                                           44HHT 8, FIZ.5, 5K, BHMARGIN 8, FIZ.5, SK
FIZ.5 / 7FIZ.5)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                    JASIER 1 JOER 2 J. (PWIKI, K=1, N)

250 WRITE [6,6020] (PWIKI, K=1, N)

250 WRITE [6,6020] (PMIKI, K=1, N)

250 WRITE [6,6020] (CMIKI, K=1, N)

260 DO 270 K = 1, N

270 H(K) = H(K) 011, + PMIKI, FWILN)

280 MT = 0.0

CALL WEIGHT [MT, RHO, NUMEL, X, Y, Z, SLOPA)

DWT = MT / WIL - 1,

15 MASS (WT) - 01 | 640,300,300

300 DO 310 K = 1, N

16 MRINIKI, 00MT | 040,300,300

C IF FIRST SIDESTEP DIDNT WORK REDUCE STEP LENGTH

330 H(K) = H(K) + H(K) + H(K) + H(K)

340 H(K) = H(K) + H(K) + H(K)

350 DO 330 K = 1, N

360 H(K) = H(K) + H(K) + H(K)

370 DO 330 K = 1, N

370 H(K) = H(K) + H(K) + H(K)

370 DO 330 K = 1, N

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                                                                                                                                                                                                                                                                                                                                                           MRITE (6,6120) (TF12.5, SK, BHMARGIN

6120 FORMAT (SK, 1, K=1, M)

150 F (10PT - 5) 170,550,160

150 F (10PT - 5) 170,550,170

170 F (10PT - 5) 170,590,170

170 F (10PT - 1) 180,340,180

C DOTHIZATION

180 CC = ICYC + 1

180 CC = ICYC + 1
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                      C CALCULATE DERIVITIES
340 IF (IK) 380,350,380
350 IF (IPRT .EQ. 1) WRITE (6,6040)
6040 FORMAT ( 31H CALCULATION OF DERIVITIVES
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                    MUDLE 44 PS
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                    FIRSTAN IV
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00070
0072
0073
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PAGE 0003
                                                                                                         IF (IK) 330,410,390
390 SLA(IK) = (MT - MTL) / (H(IK) - HL(IK))
00 400 K = 1,M
400 SLUPE(IK,K) = (CMS(K) - CMSA(K)) / (H(IK) - HL(IK))
IF (IPRT -EQ. 1) WRITE(6,6050)(SLOPE(IK,K),R=1,M),SLM(IK)
410 CONT(4UE
                                                                                   DATE 71321
                                                                                                                                                                                                                                                                                                                                      GD TO 640 RETURN TO PREVIOUS VALUE BEFORE DESCENDING 510 DG 520 K = 1,N 520 H(K) = H(K)
                                                                                                                                                                                                                                                                                                                                                                                                                                          VERSION 3. LEVEL 4
                                                                                                                                                                                                                                                                                                                                                                           IF (IPRT .EQ. 1) WRITE (6,606.0)
FORMAT ( 11H PESCENDING )
IF (ICYC .GT. 9) FMLT = 4.0TMLT
IDS = 1
                                                                                                                                                                                      1F (154 [K] - 12) 415,410,415
415 DO 420 IJ = 1,4
420 H[J] = HL(IJ)
1F (14 - 4) 430,430,180
430 H(IK) = H(IK) + ,001
1F (154 (IK)) 440,450,440
440 IA = 154 (IK)
450 IF (158 (IK)) 460,470,460
460 IB = 158 (IK)
                                                                                                                                                                                                                                                                                470 IF (1SC(1K)) 480,490,480
480 IS = 1S8(1K)
HILC) = H(1K)
690 IF (1K - N) 500,500,180
500 ICVC = C
                                                                                                                                                                                                                                                                                                                                                                                                                  GO TO 640
STORE NEW OPTIMUM
WIRN = WI
 1 + 1101 + 1011
                                                      370 HL(1J) = H(1J)
                   NTL = NT
DO 360 K = 1,
CMSA(K) = CM
TMLT = TMLT/
                                                                                  MODEL 44 PS
                                                                                                                                                                                                                                                                                                                                                                                      0909
0077
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	•					LOCATION 000010	LDCATION 000010 000030 000000 000000	LOCATION ODOOBC	LCCA11 DN 000010 000024 000036 0001C4	LCCATION 000260 000280
PAGE 0004	U	1	,		PAGE DODS	SYMBOL. MBAND	SYMBOL THE TA TC TOPT	SYMBOL	STABOL NAST HERE CAN A STABOL	3 Y#801. J
2	•	1				LGCATTOM 00000C	LOCATION 000000 000000 000000 000000	LDCATION COOO78	LDCA110N 000500 000020 000034 006053	0000 288 000 288 000 288
	ACE			.		SIZE DODOGC Symbol WLIN	SIZE ODODE8 SYMBOL THK DUM NY ITET	SYMBOL FKC	512E 000284 SEMBOL 103V M31 M31 M31 M31 M31 M31 M31 M31 M31 M31	SYMBOL
DATE 71321	CONSTANT WEIGHT SURFACE	. 8/PMT(K)		ERT OPTIMUM VALUES OPTIMIZED LAVER THICKNESSES RJ	DATE 71321	COMN / MAP S LOCATION OCCOOR OCCOOR	LOCATION 0000000 0000026 0000026 0000000	MATE / MAP SIZE LOCATION ODGC64	0014 / MAP S LOCATION CODDED CODDED CODDEC CODDEC COCZSC	LOCATION 000264 000278
LEVEL 4 0	90	•		ZED LAVER	LEVEL 4	BLDCK / SYMBOL NLOD NEL	SYMBOL SYMBOL HT THT NX IHOLE	BLOCK / SYMBOL FYF	810CK / SYMBCL 105 17 141 141 15A	R MAP SYMBOL I IA
VERSION 3. L	WI - WIMN) 530,160,160 ULATE DIRECTION COSINES DAY (SLW,PWI,Nol,154)	110T .GT. 0) GD 10 575 * .LT. 3) GD 10 575 * .GT. 7) GD 10 580 * .GT. 7) GD 10 580 * .GT. 7) GT 10 580 * .GT. 7) GT 10 580 * .GT8) XMS		SMI TAN	VERSION 3.	COMMON LOCATION 000004 000018	COMMON LDCATEGN 000000 0000004 0000000	COMMON COMMON COOCOS	COMMON LCCATION CCCOUS+ CCCOUS+ CCCOUSC DOCCOUSC DCCCOS	SCALAR MAP 10CA7 FON SYI 000250 I
\$ b\$		N-DEE	T3 640 NTINUE ST = 1		\$ 6	SYMBOL NJMEL MSN	SYMBOL H V20FF HED IFACE	SYEGOL FRI CES	N TENEST SERVICE SERVI	SWAROL
MODEL 4	C CALC	10 0 1	240	6080 FRII 6080 FRII 6090 FRII 6090 FORM 610 HKII 640 FORM 640 FORM	IV MODEL	LOCATION 000000 000014	LECATION 000000 000002 00005C 00005C	LOCATION 009000 0000 AD	LDCATION 000000 755-114 000029 000054	LOCATION 000255C 000270
FORTRAN IV	1610	0134 0134 0136 0138 0139	0140 0141 0142 0143	0144 0145 0146 0149 0150 0151	FORTRAN	SYPBOL NUMBP NUMBP	SYMBOL S10E Y10FF XC I1V9	Sewage	C O C J D C D C J D C J D C J D C J D C J D C J D C J D C J D C J D	SYMBOL M DWT

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LOCATION 000394	LOCATION	LOCATION 000646 0007E2 0008C2	000CE2	0000CA 00102A 001130 001132 001182 001486	
S Y MB OL	SYMBOL	130 150 200 200	300 PAGE 0006	i	
LCC4 TIDW 000 390	LOCATION 0003A8	LOCATION 00060A 0007C4 000856		0000BA 0000EC2 00101E 0011A6 0012F4 00146A	
SVABJL	SYMBOL	LABEL 120 6120 190 240	280	340 380 445 510 340 340 340	FMT, DUM, TC(1.
LOCATION 00038C	.ED LUCATION 0003A4	LOCATION 000574 000740 00082A	000C44	0000	\$ 001614 BYTES DDE MAS 0 DDE WAS 0 THE WEIGHT (WI.RHO.NUMEL.X.V.Z.SLOPA) TINE WEIGHT (WI.RHO.NUMEL.X.V.Z.SLOPA) TONE / SIDE.HT. HIV. 1001. SLOPA (300) 110. HED LIZ. N.X. N.Y. 1007. ITVD. IFEC. 1HOLE. ITET. ICNT. NNEW(4) AAX/NR4.NR4.LOD.NR4.STF.NR4DIA.NREC. NRECST.NRECLD.NRI.NRZ TONE / SIDE.HT. 1007. ITVD. IFEC. 1HOLE. ITET. ICNT. NNEW(4) AAX/NR4.NR4.LOD.NR4.STF.NR4DIA.NREC. NRECST.NRECLD.NRI.NRZ TONE / SIDE.HT. 1007. ITVD. S. LOPA) DOGGN (1.X.V.Z.SLOPA) OGGN
Y HAP SYMBOL X	SUBPROGRAMS CALLED ON SYMBOL DAR	L MAP LABEL 6005 6110 180		330 370 410 500 670	TES (WI.RHO, NUMEL, X., Y. Z., SLOPA) (WI.RHO, NUMEL, X., Y. Z., SLOPA) OE, HI.THK. [3.0] OE, HI.THK. [3.0] OE, HI.THK. [3.0] OE, HI.THK. [3.0] OE, HI.THK. [3.0] OE, HI.THK. [3.0] OE, HI.THK. [3.0] OE, HI.THK. [3.0] OE, S. SLOPA) OE, Z. SLOPA) C. = Z TO T1
ARRAY MAP LOCATION SI 000280 X	SUBP LCCATION 0003A0	LABEL LOCATION 0004E4 00069C 000812 00092E	000036 VERSION 3.		S 00161 & BVTES DDE MAS 0 THE WEIGHT (MT, RHO, NUMEL, X, Y, Z, SLO ION X(300), X(300), X(300), SLOPA(300) LIN X(300), X(300), X(1300), SLOPA(300) LIN X(300), X(300), X(1300), X(100) LIN X(300), X(130), X(100) LIN X(300), X(100), X(100) LIN X(300), X(100) LIN X(300), X(100) LIN X(300), X(100) TAX X/NK, NK LOD, NK SLOPA) DOG N (1, X, Y, Z, SLOPA) DOG N (1, X, Y, Z, SLOPA) DOG N (1, X, Y, Z, SLOPA) DOG N (1, X, Y, Z, SLOPA) TO DOG N (1, X, Y, Z, Z, Z, Z, Z, Z, Z, Z, Z, Z, Z, Z, Z,
SYMBOL	SYMBOL WEIGHT	LABEL 6020 6100 170	240 PS	320 360 6050 440 540 530 600	SUBGOLIA BY SUBGOLINE WEIGHI SUBGOUTINE WEIGHI SUBGOUTINE WEIGHI SUBGOUTINE WEIGHI L *KCIII)* HEDILS COHMON/RAX/MR4.NR DI NENSION ALI31*A IF (ITET) SOSSOS CALL NODGEN (1, X) GOTO 70 CALL NODGEN (1, X) HT = 3.0 CALL NODGEN (1, X) MR2 = 1 F (ITET) SOSSOS CALL NODGEN (1, X) MR2 = 1 F (ITET) SOSSOS CALL NODGEN (1, X) MR2 = 1 F (ITET) SOSSOS CALL NODGEN (1, X) MR2 = 1 F (ITET) SOSSOS CALL NODGEN (1, X) MR2 = 1 F (ITET) SOSSOS CALL NODGEN (1, X) MR2 = 1 F (ITET) SOSSOS MR2 = 1 F (ITET) SOSSOS MR2 = 1 F (ITET) SOSSOS MR2 = 1 F (ITET) SOSSOS MR2 = 1 F (ITET) SOSSOS MR2 = 1 F (ITET) SOSSOS MR2 = 1 MR2 = 1 MR3 = 1 MR4 = 1 MR5 = 1 MR5 = 1 MR5 = 1 MR6 = 1 MR7
LOCATION 000284 000398	LOCATION 00039C	LOCATION 000406 00065A 0007FA		00001C 0000F8 0000F1 00106C 001208 00135C 00138C	HIGHEST SEVERITY CEEKEC FORTRANISCO - MEEVEN CONTROL OF
SYMBOL SLOPA SLOPA	SYMBOL	LABEL 6000 140 160 210	250 FORTRAY	310 6040 6040 6040 6060 6060 6060	COMPLER H COMPLER H OD01 OD02 OD03

				L OCATION 000020 00005C 00005C	LOCATION 000010	LOC ATI ON 00010C 000120 000134 000148 00015C	LOCATION 00019C	LOCATION
		Z 0003	E 0003	SYMBOL Y10FF XC 1 TYP NNEW	SYMBOL	SY BOL 18 ELGT 11JK XXP XXP XXP XXP XLGT	SYMBOL	SYMBOL
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				LOCATION 00001C 000030 0000C0	LOCATION 0000000 0000020	LOCATION 000108 000111 000130 000158 00016C	LOCATION 000198	LOCATION 0001DC
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210VRP + AL	.28867 + YIOFF .86657 + Y2OFF (HWT-1)-Y2LGT + YILGT (FLOATHW)5)-SIDE /(YLGT-2K,GT) 6.6100) WT	VERSION 3.	VERSTON 3.	COMMON COCATION 000004 0000028 000000000000000000000000	CDMMON LDCAT10M 000004 000018	SCALAR 10C ATI ON 000 100 000 128 000 136 000 164 000 178	ARR AV ON	SUBPROC LOCATION 000104
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NUMMP = 2**X**NY
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Y2LGT = .86657 + V2DFF
DO 300 I = I,NJHNP
IROW = (I-1)/NX + I
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			PAGE 0002			
IF (ICDL) 80,70,80 70 ICDL = NX	""0""0""	LECTION TO THE COUNTY OF THE C	C GENERATE CYLINDER COORDINATES 350 CISC = V2(GT0510ENK 4.28318 RAD = V2(GT0510ENK 4.28318 IF (IDPT .ME. 1) MRITE (0.6200) RAD 6200 FORMAT (.//, slow, 17HCYLINDER RADIUS 8 , F15.5) 6000 FORMAT (.F15.5) DO 400 I = 1, MJMMP MODEL 44 PS VERSION 3, LEVEL 4 DATE 71321	THETA = [V(1)/CIRC)+6,2831 A9C = Z(1) V(1) = [RAD-A9C)+COS(THETA 400 Z(1) = [RAD-ABC)+SIN(THETA 450 CONTINUE GD TO 800	G GENERATE AIRFOIL COOMDINATES 550 CONTINUE CHORD = Y2LGTeSIDEENX/2. IF (10PT .NE. 1) WATTE (6.6250) CHORD 6250 FORMAT (/// .10x, 15HAIRFOIL CHORD 6 . F15.5)	DO 640 [= 1.MUMNP FACT = 1.0 IF (171 = 2.0CMD10 = 10.410.400 810 XOC = 1.0 81
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	LOCATION	LOCATION 000020 00005C 0000C4	LDCAT10W 000178 00018C 0001A0	LOCATION	LOCATION 0004-02 0006-0C 00095A 00095A 0008-0C 0008-0C
	PAGE 0003 SYMBOL MBAND	SYMBOL V10FF XC I TYP NNEW	SYMBOL I AOM CHORD ZOLD	S Y MB OL	LABEL 95 300 6030 605 605
1 TAE 3	LUCATION OCCOOL	LOCATI ON 000015 000005 0000050	LOCATION 000174 000188 00019C	LOCATION 000184 LOCATION 0001C4	L DCA 710N 0003DA 0005AO 0007CB 000900 000AAC
	SIZE 00006C Symbol MLIN	\$12E 0000EB \$7MB3L THETA TC TO TOP?	S YMBOL I ABC SLOP	SYMBOL SLOPA SYMBOL CDS	LABEL 2 90 2 50 6 2 50 6 2 50 6 2 50 6 2 50
•	321 00 00	DNE / NAP : LDCATION D00008 000000 000000	10CAT10N 000170 000184 000198	LDCAT10N 000180 LED LCCAT10N 0001C0	LDCATION 0003C6 000546 000546 00057C 00089C
630.630.635 1) /SLOPA(I)) MN	BLOCK / SYHBOL NLOD	BLOCK / SYMBOL THX DUM NY ITET	RAP SYNBOL V 2L GT R AD T DC	ARRAY MAP ON SYMBOL Z SUBPROGRAMS CALLED ON SYMBOL SIN	MAP LABEL 185 200 350 350 850 640
	CONING	COMMON CO	SCALAR 10CATION 00016C 000180 000194	ARRAY LOCATION OCOLAC SUBPROY LOCATION COOLSC	1.48EL
IF (FACT) 624,622,622 IF (2(1)) 625,640,640 IF (2(1)) 640,640,629 Z(1) = 0.0 Z(1) = 0.0 IF (V(1) = CHORD/2.) Y(1) = AS(H1/5L0PA(1) CONTINUE CONTINUE IF (IMT - EQ. 1) RETU CONTINUE IF (IMT - EQ. 1) RETU RETURN END	5 PS SYMBOL NUMEL	SYMBOL HT THT NX IHOLE	SYMBOL V115T CIRC XOC	SYMBOL SYMBOL GURY	LABEL 150 150 6150 620 620 635 EMENTS 000C
622 1 624 1 625 2 625 1 630 4 630 4 640 C 640 C	MODEL DCATION 00000	000000 000000 000000 000000	LOCATION 000168 00017C 000190	LOCATION 0001A8 LOCATION 000158	LOCATION LABEL LCCATI 000326 80 00034 00047C 150 000408 00085 6150 000408 000862 650 000896 000986 620 000970 00082 630 000870
0066 0066 0066 0071 0072 0074 0074 0074	SYMBOL L	SYMBOL SYMBOL Y ZOFF HED I FACE	SYMBOL NYA I COL FACT I WRT	SYMBOL X SYMBOL 18COMB	LABEL 100 6140 400 630 107AL M

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COMPILER MIGHEST SEVERITY CODE MAS O //PLATEN EXEC FORTRAMIBCO, MAP)

COMPANIENT PS VERSION 3, LEVEL 4 DATE COMMON / CONY / VUNNE, WILS / LIFE COMMON / CONY / VUNNE, WILS / LIFE COMMON / PALT / MAIL 100 / LIFE COMMON / PALT / MAIL 100 / LIFE COMMON / PALT / MAIL 100 / LIFE COMMON / PALT / MAIL 100 / LIFE COMMON / PALT / MAIL 100 / LIFE COMMON / PALT / LIFE COMMON / PALT / LIFE COMMON / PALT / LIFE COMMON / PALT / LIFE COMMON / PALT / LIFE COMMON / PALT / LIFE COMMON / PALT / LIFE COMMON / PALT / LIFE COMMON / PALT / LIFE COMMON / PALT / LIFE COMMON / PALT / LIFE COMMON / PALT / LIFE NECTO I SO I SO I SO I SO I SO I SO I SO I	71321 PAGE 0001 .	### ##################################				
	44 PS VERSION 3. LEVEL	SUBROUTINE PLATSN SUBROUTINE PLATSN SUBHON / SONN / NE / SI 1 "XC(11)" HES(12) COMMON / MAIL / E COMMON / PLAT / NE COMMON RAX/MRG-MRG-MRG-MRG-MRG-MRG-MRG-MRG-MRG-MRG-	GENERATE VERTICAL NAGOD = NX/2 + 1 NBODD = SNX/2 + 1 NBODD = SNX/2 + 1 NBOD = SNX/2 + 1 NEVN = 1 NBEVN = 2 NX + 1 NBEVN = 2 NX + 1 NP = 0 NK = 0 NK = 0 NK = 0 NK = 0 NK = 0 NK = 0 NK = 0 NK = 0 NK = 0	NK = NK + 1 1F (NK - 1) 90.10 90 NA(NP) = NEVN + NC(NP) = NEVN + NC(NP) = NEVN + NC (NP) = NEVN + NC (NP) = NEODO + NC (NP) = NEODO + NC (NP) = NEODO + NC (NP) = NEODO + NC (NP) = NEODO +	MPIR = 201N-11 NO 300 1 = 2-NPIR NP = NP + 1 NA(NP) = NC(NP-1) NC(NP) = NG(NP-1) 300 NS(NP) = NA(NP) + 1 NAM = 1 NAM = 1 NAM = 1 NAM = NX + 1 NAM = NX + 1 NAM = NX + 1 NAM = NX + 1 NAM = NX + 1 NAM = NX + 1 NAM = NX + 1	NEB = 30NK NCB = 50NK/2 NCB = 60 NK/2 NG (NP) = NA NG (NP) = NA NG (NP) = NA NG 00 1 = 2.0 NG N NF = 1 NF = 1

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NA BANGE NA	MB(MP) = MC(4P) + 3+MX/2 GO TO 650 MB(MP) = MC(MP) - MX/2 + 1 GO TO 650 MB(MP) = MC(4P) - MX/2 COVITIVE IF (I. CE, MX/2) 4PIR = MPIR + 4 IF (I. CE, MX/2) 4PIR = MPIR + 4 IF (I. CE, MX/2) 4PIR = MPIR + 4	If ii. EQ. MX/2) MPIR = MPIR + 1 21P UP SEAF OF CYLINDER IF (ITVP - 2) 740,710,710 DO 730 I = 1.M MM in P + 1 MM (MP) = MGA + (I-1)**MX-2 HG(MP) = MGA + (I-1)**MX-2 HG(MP) = MGA + (I-1)**MX-1 IF (I - MY) 720,730,720 MP = MP + 1		GENERAL CHENERAL = NX - 1		
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FORTRAN IV
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	PAGE 0004						PAGE 0007	SYMBOL. MBAND	SYMBOL 410FF XC 117F NNEW	SYMBOL FYC	SYMBOLINT
	•	LEEN	TAPE 3	1496.2	100.3		•	1004 T 1 ON 00000C.	LUCA T13N 00001 C 00003 0 0000C0 0000C0	LDC ATT ON 000078	LUCATION 003390
		15. 10H HAVE BEEN	E33, THICK, J	0 J = 1.NUMEL 0 J = 1.NUMEL • NR2) I. MA(I) - MB(I) - NC(I) - MD(I) - E22-E11-E12-E39-THICK-IMIL	EQ. NNEW(K) K = K + 1 THK(K) 3) I.MA(I),MB(I),NC(I),ND(I),E22,E11,E12,E33,THICK,IMT.			SIZE 00006C SYMBJL NLIN	SIZE 0003EB SYMB3L THE TA TC 10PT ICNT	SIZE ODDODC SYMBOL FXC	512E 0044E0 SYMBOL NST
	DATE 71321	NOE.	= MOCH = THK(1) [3) I,MA(I),MB(I),MC(I),MD(I),E22,E11,E12,E33,THICK,J 1020 UE).622.611.612	.E22,E11,E12,		DATE 71321	COMN / MAP LOCATION 000008 00001C	DNE / NAP LOCATION 000008 00002C 0000BC 0000D0	HATE / HAP LOCATION 000064	PLAT / MAP SIZE LOCATION 002260
66 67 67 67 67	LEVEL 4	TES CONNECTING TO ESS TO SIMULATE A	VC(1). ND(1)	, NC(1), MD(1	(+ 1 (C(1),ND(1)		LEVEL 4	BLDCK / SYMBOL NLOD NEL	BLOCK / SYMBOL THK DUM NY 17ET	BLOCK / SYMBOL FYT	DN BLOCK / PLAT SYMBOL L NC NC
E(2,1)/XNU E(3,1) E(3,1) OLE-1) 890,860,860 (1 1-NOMH) 865,885,865 (1 1-NOMH) 870,885,870 (1 1-NOMH) 890,885,870	VERSION 3.	6.6150) NODH (/// 5x, 25HPLATES GIVEN ZERD STIFFNESS	AALI J. NB(I) .	1. NUMEL	.EQ. NNEW (K) K = K + 1 THK (K) (3) I NA(I) NB(I), NC(I)	6 6 F1 5 - 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	VERSION 3.	COMMON LDCATION C00004 000018	COMMON LOCATION 000004 00002 8 0000 86	COMMON LOCATION 000050 000084	CORPON LOCATION 001130
E12 = E(2,1)/XNU E13 = E(2,1)*E(4 E33 = E(3,1)*E(4 IF (IMCLE-1) 890 IF (MC(1)-NOOH) IF (MC(1)-NOOH) IF (MC(1)-NOOH)	44 44 E22 E22 E22 E22 E22 E22 E22 E22 E2	o ~	IMOLE = NODM THICK = THK(1) WRITE (3) I,NA GO TO 1020	K = 1 DO 1010 J = NRZ=J READ(2*NRZ)I	IF (J .EQ. NNE THICK = THC(K)	- 2 -	r :	SYMBOL NUMEL MBN	SYMBOL HT THT NX 1HOLE	SYPBOL FXT CHS	SYMBOL
0900	IV MODEL	6150	900		1010	1020	IV MODEL	LOCATION 000000 000014	LOCA TEON 000000 000024 000088 000088	LOCATION 000000 000000	LOCAT FON 000000 003380
2220 1220 1220 0220 0220 9120 9120	604 TRAN 0224 0225	0227		0234 0235 0236 0231	0238	00243	FORTRAN	SYMBOL NURNP NUPBLR	SYMPOL SICE Y20FF MEO IFACE	SYMBOL E SSS	S YMBOL NA ND

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LOCATION 000010	LOCATION 000228 000235 000235 000256 000278	LOCATION LOCATION 000 600 000 882 000 882	000 ED6 00112E 0013A2 001672 00170E	
SYMBOL	SYMBOL NBEWN NP 18 NAB NOS THICK	SYMBOL LABEL 310 500 540 540	72 0 0008 72 0 72 0 72 0 72 0 72 0 72 0 72 0 72 0	P AGE 8001
LOCAT FON 00000C 0000 20	LOCATION 000238 000238 000246 000246 000274	LOCATION LOCATION 0004C8 0007A4 0009E0	000E44 0010E2 00134E 001720	
SIZE 000024 SYMBOL NR4DIA MR2	SYMBOL NAEVN NV HCA J NODH E33	SYMBOL LABEL 300 540 540 610	710 770 605 6200 6150	K. M. M. MEL (2) IM. D. D. M. MEL (2) IM. D. D. M. MEN T. J. D. M. D. M. S. M.
RAX / MAP S LOCATION 000006 00001C	LOCATION 000220 000234 000248 000256 000270 000284	LOCATION LOCATION 000466 000714 000974	DATE 71321 0000F0 000F76 001316 0014C2 0016DE	
BLOCK / SYMBOL NR4 STF NR1	R MAP SYMBOL NCODD NK NBA NOSA NHPT E12	SUBPROGRAMS CALLED ON SYMBOL LABEL MAP LABEL MAP LABEL ON LABEL 30 50 50 50 50	1 EVEL 4 700 750 850 850 1010	LFVEL 4 51.074) 51.074) 61.1776 100.1779 100.1779 100.1779 100.1779 100.1779 100.1779
COMMON LOCATION 000004 000018	SCALAR LOCATION 000230 000230 000244 000258 000260	SUBPR LOCATION LOCATION 000416 000706 00096A	VERSION 3. 000D2E 000F34 0011FA 0016F8 001686	### ### #### #########################
SYMBOL NR41.00 NRECLD	SYMBOL NBODD MP NAA NCB RCB E 2 2	SYMBOL LABEL 100 5000 910	65 650 740 830 830 870 1000	FREMENTS OD1ABE FRITY CODE WAS FREED, WAP) L 44 PS FREED COMMON COMMON COMMON COMMON COMMON COMMON COMMON COMMON COMMON COMMON COMMON COMMON FREED COMMON COMMON FREED COMMON
LOCATION 000000 0000 14	LCCATION 000218 00022C 00024C 00024C 00026E 00027C	LOCATION 000294 LOCATION 0003A0 00065E 000862	1V MODEL 000CFC 000F3A 0011D0 0013EE 0016%	ICMOLY REQUI
STHBOL NR4 NRECST	SYMBOL NADDD HCEVN I NBB NY 1 E 1 1 I NTL	SYNBOL IBCOME LABEL 90 305 505 505	FORTMAY 630 730 730 790 869 900	COMPLIER W/LOGGEV E PORTRAN PO

			PAGE 0002		
MYND = 2*NY + (2*NY-1)* Y167 = .28867 + Y10F X167 = (1.0K-1)**2157 + X167 = (1.0K-1)**2157 + IUNIT = (F.LOA7(NY)5)**5] F (1.0K-2) = .001) F (1.0K-2) = .001) F (1.0K-2) = .001)	S (YPON) YP 2) 82, YPON) YP 2) 82, XYPON) S (XYPON) S (XYPON) S (XYPON) S (YOSHR) -	FLAT PLATE END LOADS HX 2 PX	44 6 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	HXV HXV PXVB = XYLOD=XLGT/NXND PXVB = XYLOD=YLGT/NYND DO 200 I = 1, N4 UY (1) = UY (1) - PXVA UY (1) = UY (1) - PXVA UY (1) = UY (1) - PXVA UY (1) = UX (1) - PXVA IA9 = 2**NX**I - I) UK (NI) = IA9 = UX (NI) - IA9 - UX (NI) - IA8 - UX (NI) - UX (N	250 UK(N4+1AB) = UK(N4)1AB) + PKVB 1F (1UNIT) 70,70,800 PV 270 PVV = VHOM *VLGF/(Hf *MV) DO 300 I = 1, hV IAB = 2*NK**(I-1) UV(N1+1AB) = UY(N1+1AB) + PWV UV(N3+1AB) = UY(N2+1AB) - PWV UV(N3+1AB) = UY(N3+1AB) + PWV UV(N3+1AB) = UY(N3+1AB) + PWV
0018 0018 0020 0021 0023 0025 0025		0035 102 0034 103 0034 103 0034 103	A	0050 0051 0051 0053 0055 0055 0055	

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) DATE 71321
PPX = XMOY/SUM/3.

DO 345 I = 1.44

UN(1) = UX(1) + PHXeZ(1)

345 UX(1+CONST) = UX(1+CONST) - PHXeZ(1)

350 IF (1UNIT) 80.80.80.80

360 PHXA = XVMONS VLGT/(HTONY)

PHXY8 = XYMONS VLGT/(HTONX)
                                                               VERSION 3. LEVEL 4
                                                                                                                                               MODEL 44 PS
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                                                                                                                                                                                                                                                                                                                                              PL = VQS-ReVLGT/NVND
PMX = YQS-RReSIDE = 1.86657+V2OFF) = VLGT/12. = HT 647)
DO 620 I = 1. AY
UZ(1+3+NX/2) = UZ(1+3+NX/2) + PZ

UX(1+CON ST ) = UX(1+KONST ) +

UZ(1+KONST + NX/2) = UZ(1+KONST + NX/2) -

UZ(1+KONST + NX/2) = UZ(1+KONST + NX/2) -

UZ(1+KONST + NX ) = UZ(1+KONST + NX/2) -

UX(1+KONST + NX ) = UZ(1+KONST + NX/2) -

UX(1+KONST + 3+NX/2) = UZ(1+KONST + 3+NX/2) -

EVX(1+KONST + 3+NX/2)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                             GENERATE TORSION LOADS ON AIRFOIL

ARM = 0.0

AD 750 I = 1.N4

AN 5 = 5.10PA(1)

ARM = AFM - YIII0SIN(ANS) + Z(1)0COS(ANG)

PA = TORQ/ARM/3.
                                                                                                                                                                                                                                                                          . VERSION 3. LEVEL 4 DATE
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                              GENERATE TORSION LOADS ON CYLINDER
650 IF (1TYP = 2) 800.660,720
660 RAD1 = SQRT(YII)**0.242(110**2)
RAD2 = SQRT(YII)**0.242(110**2)
PA = TORQ/(NX*0RAD1*RAD2**0.2/RAD1)**0.3
PA = RAD2/RAD1*PA
PA = RAD2/RAD1*PA
DG 700 I = 1,**6
THETA = ATAN2(Z(I),*Y(I))
SINITH = SINITHETA*1.57079)
CGSTH = COS(THETA*1.57079)
PX = PA
IF (I = GT - NX/2 - AND, I = LE. NX) PX = PB
UV413 = UV413 + PX*COSTH
UZ413 = UV413 + PX*COSTH
UZ413 = UV413 + PX*COSTH
CG 10 800
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                              If (2(1) ... T. 0.0) ANG = ANG + 3.14159

UV(1) = UV(1) + PA=COSTANG)

UV(1) = UV(1) + PA=SIN(ANG)

UV(1+KONST) = UV(1+KONST) - PA=CDS(ANG)

UZ(1+KONST) = UZ(1+KONST) - PA=SIN(ANG)
                                                                                                                                                                                                                                                                                                                                                                                                                   MODEL 44 PS
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				LOCATION 000010	LOCATION 000020 0000350 000006	LCCATION 000010	LOCATION	LOCATION 000188 0001CC 0001E0 0002 CG 0002 CG	LOCATION LOCATION 000254
PAGE 0005			P AGE 0006	SYMBOL MBAND	SVHBOL V10FF XC HTVP NNEW	SYMBOL	SYMBOL	SY MOOL N3 Y1L GT PX PHY PZ PB	SYMBOL SYMBOL COS
•	18E 3	TAPE 3	•	LOCATION	LOCA TI ON 00001 C 00003 0 0000 C 0000 D 4	LCC A TI CN 00000C 000020	LOCATION 000E10	LOCATION 000184 000106 00010C 0001F0 000204 000220	L DC ATI DN 000 240 L DC ATI DN 0002 50
				S12E 00006C SYMBOL NLIN	\$12 € 0000€ 8 \$74831, TMETA 1C 10PT	SIZE 000024 SYMBOL XMON YQSM	\$12E 0012C0 SYMBOL UZ	SY483L NYA NYA JUNIT PAKE PAKE PAKE PAKE PAKE	SYMBOL SLOPA SYMBOL SIN
DATE 71321			DATE 71321	COMN / MAP SIZE LOCATION 000008 00001C	LOCATION 000008 00002C 00008C	LODA / MAP LOCATION 000008 00001C	LODS / MAP LOCATION 000960	LDCATION 000180 000180 0001C4 0001EC 000200 000214	LOCATION 00023C LED LOCATION 00024C
LEVEL 4	OLE) # 7.		LEVEL 4	BLOCK / SYMBOL NLOD NEL	SYMBOL SYMBOL THE DUM NY ITET	BL DCK / SYMBOL KYLOO XQSHR	BLOCK / SYMBOL UV	R PAP SY480L N1 NXND YLGT PXYA PXYA RADZ NEND	ARRAY MAP ON SYMBOL 2 SUBPROGRAS CALLED ON SYMBOL ATAN2
VERSION 3.	COVIINUE MRITE (3) (UK([],UV([],UZ([]),[=],MUMMP) DO 900 1 = 1,MUMMP CODE([] = 0.0 CODE([] = 0.0 CODE([] = 7. CODE([] = 3.	7) = 6. (3) (CODE(1), 1=1.NUMP)	VERSION 3.	COMON LOCATION OGGOG4 OGGOIS	CONMON LCCATION 000004 000026 000008	COMMON LOCATION 000004 000018	COMMON LOCATION 000480	SCALAR LOCATION 0001AC 0001D4 0001FC 0001FC 000210	ARRAY WAP LOCATION S 000238 Z SUBPROGRA LOCATION S 000248
:	CONTINUE WATE (3) (DO 900 1 = CODE(1) = CODE(1) = CODE(1) = CODE(14) = CODE(CODE(17) = WAITE (3) (RETURN END	\$4 :	SYMBOL NUMEL NON	SYMBOL NT THT NX I HOLE	SYMBOL YLOAD TORG	SYMBOL	SYMBOL I KONST KLGT 1AB SUM COSTM	SYMBOL SYMBOL SORT
MODEL	000		MODEL	LOCAT 10M 000000 0000 14	LOCA T 1 ON 000000 000024 000008	LOCATION 000000 000014	LDC A T 1 ON	LOCATION 000186 000186 000100 000164 000168 000200	LOCATION 000234 LOCATION 000244
FORTRAN IV	0172 0173 0174 0175 0176 0177	0179 0180 0181	FORTRAN IV	SYMBOL NUMNP NUMNP	SYMBOL SIDE V 20FF MED IFACE	SYMBOL	SYMBOL	SYMBO! NN 3 NA 4 V2LGT V2LGT VALA VALA VALA VALA VALA VALA VALA VAL	SYMBOL II

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	LABEL	2	£ §	520	350	PAGE 0007		450 400 400			PAGE 0001	•																			1					
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	LABEL	•	105	200	325		828	5 8				••••••	IK.NON.NEL	141, OUT, TC	THE PART OF THE PARTY OF THE PA				i			1/TMT	}								•					
	LOCATION	3000	000000	0007F2	0000 TA	DATE 71321	901100	00100	•		DATE 71321	sessessessessessessessessessessessesses	JOHN (IMPT.X.Y.Z.SLOPA) / NUMP.NUMEL.NLOD.NLIM.NBAND.NUMBLE.NBN.NEI(20)	/ ONE / SIDE, HT, THK(S), THEIA, VIOFF, VZOFF, THT, DUM, TC(111)	A(300)							.57734+Y10FF)+S1DE+HT/THT I/T M	•			i	i									
₹,	LABEL	2 :	103	170	348	LEVEL 4	9	900			+ LEVEL 4	DVER	JAE L. ML 00.NL	THE STAFF	213001.510	PLATE NODAL COORDINATES		9	SIDESTANTH	7-10	/THT	: - (.57734	(F)				_			(II+I)+SINTH	+ Y([]+2)					
LABEL	10CA 10M	45 5000	0000	9600	00000	VERSION 3.	00118A	001 644 00 1C9E	OOLESO BYTES	0 5	VERSION 3.	***************************************		1 SIDE, HT.	ON X(300), Y(300), Z(300), SLOPA(300)	IT PLATE NOD	*C(11) + .00001	STY (THE TA/57.2958)	(.28667 + YIOFF) SIDE TANTH	(I-ANOEJOXN + ANOXN	10E/2. + XOFF) • HT/THT	86603+ V2OFF) + SIDE - (.5773	-	.86503 + Y20FF	# 1.MX		+ VINC+([-])	z.	* ([[+])	/2. + VIII+1	SIDE/2. + Y(I)		o			
	TABEL	2	102	051	346	EL 44 PS	450	650 760	REQUIRENENTS OUT	VERITY CODE MAS	SEL 44 PS		COMMEN / COMM		DIMENSI	4	NOTE - 48NY		XOFF - (.28		- 5	Y1 = (.86603 Y2 = (.28867	¥	72LGT = .866 IK = 0	300 1		Y(11) = Y1 +			-		Ξ	2(11+1) = HI 2(11+2) = 0.0	•		10
	491000	D00051A	0000 9A	0006AA	- 00000	IN IV MODEL	00105E	001850	HEMORY	HISHEST SEVERITY CO EXEC FORTRANIBED, MA	IN IV MODEL					ں د							;			110				1						
1986	200	2	5	110	335	FORTRAY	004	6 20 750	TOTAL	COMPILER //NODGAV	FORTRAN IV		0000 0000	0003	4000		\$000 0000	0000	6000	0100	2100	0013	0015	0016	8100	02 00	1700	0053	\$200	0056	0024	6200	1600	2600	0034	0035

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DATE 71321
                                                                                                                                X(II) = X(II-4) + SIDE

Y(II) = Y(II-4)

300 Z(II) = Z(II-4)

IF (IDPT NE. I) WRITE (6,6150) X(NODLG)

6150 FORMAT ( // 1,14x, BHLENGTH # , F15.5 )

IF (ITYP - 2) 800,350,550
                                                                                                                                                                                                                                                                    CHORD = Y2LGT #SIDE#NX/Z.
IF (IOPT .NE. 1) WRITE (6.6250) CHORG 6250 FORMAT ( /// .10x, 15HAIRFOIL CHORD
VERSION 3. LEVEL 4
                                                                                                                                                                                                                                                                                                                                               KOC = Y(I)/CHORD
CALL CURV (KOC, TOC, KC, TC, SLOP)
SLOPA(I) = SLCP*FACT
                                                                                                                                                                                                                                                                                                      DO 640 I = 1, NJMNP
FACT = 1.0
IF (YII) - CHCRD) 610,610,605
Y(I) = 2,0CHORO - Y(I)
                                                                                                                                                                                                                                                   GENERATE AIRFCIL COORDINATES 550 CONTINUE
MODEL 44 PS
                                                                                                                                                                                                                                                                                                                                                                                                               622
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				L OC AT I ON 000010	LUCAT 1 DN 00002 0 00005 C 000005 C	LOCATION 000164 000178 00018C 000180	LOCATION	1.0CAT10N 0001.04	LCCATION 0006FC 0008FC 0009EC 000ACE	
PAGE 0003			PAGE 0004	SYMBOL	SYMBOL Yldff XC Ityp Nnew	SYMBOL XI Y2LGT CIRC KOC	SY MBOL	SYMBOL	LABEL 6150 620 635 635	
•	0	TAPE	•	LOCATION	LOCATION 00001C 000030 0000C0 0000D4	LOCATION 000160 000174 000188 000190	LOCATION 0001CD	LOS ATION 000100	LCCATION 000684 00085E 000962 000A9E	
		1		SIZE 00006C SYMBOL NLIN	SIZE 0000EB SYMBOL THETA TC IOPT ICNT	SYMBOL XOFF YINC J FACT IMRT	SYMBOL	SYMBOL	1ABEL 333 400 610 630	
DATE 71321		2	DATE 71321	COMN / MAP SI LOCATION 200008 COSOIC	ONE / MAP SI LOCATION 000008 00000C 00000C	LOCA TION 00015C 000170 000164 000198	LOCATION 000 1BC	ED LOCATION 0001CC	LOCATION 00063C 0007 E6 000956 000468	
3, LEVEL 4) . I - I , MJMN	LEVEL 4	BLOCK / SYMBOL NLOD NEL	BLOCK / G SYMBOL THK DUM NY ITET	MAP SYMBOL TANTH Y2 I1 CHORD ZOLD	NAP SYMBOL Z	SUBPROGRAMS CALLED ON SYMBOL S?V	MAP LABEL 150 6000 605 625 625	
VERSION 3.	1) = ASS(HT/SLOPA(I)) 10 = G40 1) = CHORD - ABS(HT/SLOPA(I)) MIINUE NIINUE PMAT (60x, 104MUDAL DATA) 500 1 = 1,MUNNP FMAT (14,20x,4f12.5)	IF (INT .EQ. 1) RETURN WRITE (3) (1, X(1), Y(1), Z(1), I-1, MJHNP) RETURN END	VERSION 3.	COMMON L DCATI UN 000004 000018	COMIDN COCOD+ COCOD+ COCOB6 COCOE	SCALAR LDCATION 000158 00016C 000180 000194	ARRAY LOCATION 000188	SUBPRO LOCATION COOLCE	LABEL LCCATION 00052E 00075C 0008FC 00084B	SC BYTES
4	7(1) = AB CO TO 640 CONTINUE CONTINUE FORMAT (DO 500 I CONTINUE CONTINUE	IF (INRT .EQ. WRITE (3) (I; RETURN END	24 44	S Y MB OL N UM EL NB N	SYMBOL HT THT NX HOLE	SYMBOL SINTH Y1 I ABC SLOP	SYMBOL	SYMBOL	LABEL 120 6250 6250 624 800	IREMENTS DODGEC BYTES
×	6 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	u	IV MODEL	LOCATION 000000 000014	LOCATION 000000 000024 000088	LDCATION 000154 000168 000176 000190	LOCATION 000184	LCC ATION 0001C4	LOCATION 0003D0 000734 000898 000A22	MEMORY REQUIRE
FOR TRAN IV	7 # # # # # # # # # # # # # # # # # # #	0101	FORTRAN	SYMBOL NUMBER NUMBER	SYMBOL SIDE YZOFF MED IFACE	SYMBOL 43DLG K2 IK RAD TOC	SYMBOL	SYMBOL	148EL 110 350 550 640	TOTAL M

COMPILER MIGHEST SEVERITY CODE WAS O //PLATGA EXEC FORTRANIBCO, MAP!

## PAGE 0001 *********************************						PAGE 0002
**************************************	AEVN = 1	NOTES 44NV 1 1 1 1 1 1 1 1 1		MARAP = MST(2) + MLEG*(1-2)/2 NK = 3 NB(NP) = NA(NP) + INT(NK) ND(NP) = NR(NP) + INT(NK+1; ND(NP) = 0 ND(NP) = 0 ND(NP) = 0 ND(NP) = 0	~~~ <u>~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~</u>	FINALE SKEW A LE PS VERS ((1) = NP + 1 ((1) = NP + 2 (1) = 4+4V (1) = 4+4V (1) = 4+4V (2) = 4+4V (3) = 111 = 4+4V (4) = 111 = 4+4V
	u ,		210	220	5 50 5 50	NO DEL

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VERSION 3. LEVEL
                                 - NST(2) + (1-NX/2-1)+
                                                                                                 NOS - (4-NY-110 (NX-17 +3
                                     370 NBINP
                                 360
                                                                                                     FORTRAN IV
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ND(NP) = 0

IF (NK .EQ. 1) 50 TO 650

IMOND) = NC(NP) + INT(NK+2)

CONTINUE

IF (I .GE . NX/2 . AND. I .GE. NV) .CR. (I .GE. NV) .NPIR = NPIR + 1

IF (I .EQ. NV) NPIR = NPIR + 1

IF (I .EQ. NV) NPIR = NPIR + 1

IF (I .EQ. NX/2) NPIR = NPIR + 1

IF (I PC. CONTINUE

CONTINUE

IF (ITYP .EQ. 1) GO TO 740

ZIP UP SIDE DF CYLINDER

NOA = 3
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 DATE 71321
                                                                                                                                                                                                                                                                                                                                                                                                                      GO TO 570

GO NA(NP) = NST(2) + (1-NX/2-1)++

NK = 3

TO NB(NP) = VA(NP) + INT(NK)

NC(NP) = NB(NP) + INT(NK+1)

NC(NP) = 0

IF (NK .EQ. 3) ND(NP) = NC(NP) + INT(NK+2)

DO 650 J = 2. NPIR

NP = NP + 1

NK = NK + 2

IF (NK .EQ. 5) NK = 1
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                       CONTINUE
NUMEL = NP
MRITE 16,6200) NAEVN, NNEW(1), NNEW(2)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 VERSION 3. LEVEL 4
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                   610 NAINP) = NCINP-1)

IF (NK .EQ. 1) NAINP) = NDINP-1)

620 NBINP) = NAINP) + INTINK)

NCINP) = NBINP) + INTINK+1)
NOS = (40NY-1)0(NK-1) + 1

NO 730 F = 1.NY

NOT NO + 1 = 1.NY

NOT NO + (1-1)04

NOT NO + (1-1)04

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				LOCATION 000010	LOCATION 00002C 00005C 0000C4
	•	PAGE 0005	1	PAGE 0006 SYMBOL MBAND	SYMBOL V10FF XC 11YP NNEW
;		BEEN TAPE 3	NTL TAPE 2	LOCATION	LDCATION 00001C 000030 0000C0 0000D4
9 .14. SK.BHSKEN A		15. 10H HAVE } E33.THICK,J	, 633, THICK, II 633, THICK, IN	SIZE ODOGÉC SYMBOL NLIN	SIZE 0000EB SYMBOL THETA IC IOPT
		DATE 71321 ING TO NODE. LATE A HOLE),622,611,612 ,622,611,612;	COMN / HAP 5 LUCATION O00000 00001C	LUCATION 000008 000002 000000 0000000
IST VERTICAL PLATE) HW = 1 HHOLE	EC 1-1) eE (1-1) VEL 4 IS COMMECT IS TO SIMUL (11) MO(11).	CC 13,NDC 5 + 1 CC 13,NDC 13 CC 13,NDC 13 LC 1MTL 3	BLOCK / SYMBOL NLOD NEL	SYMBOL SYMBOL THE DUM NY 11E*	
, 5%, 22HFII SFEM B & 14 -1) HMX/2 + 24 I - 1) NODH =	NUMEL. 4. J. 16 (2, J.) XNU XNU E90, 860,860 OH) 655,885,89 DH) 870,885,80 DH) 870,885,80 H) 890,885,80 H) 890,885,80	S VERSION 3, LEVEL 4 DATE 71321 = NODH (6,6150) NODH (7/7, 5X, 25HPLATES CONNECTING TO NODE, 15, 10H HAV GIVEN ZERO STIFFNESS TO SIMULATE A HOLE) = THK(J) (615) (615) (1 615) (1 1.) NA(11), NG(11), NO(11), E22, E11, E12, E33, THICK, J 1020	MUMEL MA(I) NB(I) N MA(K) K N MA(I) NB(I) NA MA(I) NB(I) NB(I) NA MA(I) NB(I) NB(I) NA MA(I) NB(I) NB(I) NA MA(I) NB(I)	COMION DO COMION LUCATION COCOCA COCO	COMMGN LCCATION 0000058 0000068
ORMAT (/// 14, 5%, 8H; 10DH = (4*NY, F (1HOLE, G)	15 (1 .60. NNEW, NNU = 1. (20. NNEW, NNU = 1. (20. NNEW, NNU = 1. (20. NNEW, NNU = 1. (20. J) / XNU (5.2 = (1. J) / XNU (5.2 =	44 PS HDLE = NDH RITE (6.6150) ORATI (/// 40H GIVEN 2 HICK = THK(J) ORATI (615) ORATI (615) OR TO 1020	D	SYMBOL NUMEL NBM	SYMBOL HT THT NX IHOLE
0002 0002 0002 0002 0002		615 0 6100 6000 9000	20 00 00 00 00 00 00 00 00 00 00 00 00 0	MODEL 000ATI DN 00000	LDCATION 000000 000024 000085 000085
0169 0170 0171	0173 0175 0176 0177 0177 0178 0180 0183 0183 0184 0186	2	0197 0198 0200 0201 0204 0205 0206 0206 0206 0206	FOR TRAN IV SYMBOL NUMNP ONUMBER	SYMBOL LESTON TO THE CONTRACT OF CONTRACT

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LOCATION	LOCATION 003398	LOCATION	LOC AT I DW 000 2 20 000 2 34 0002 48	LOCATION	LCCATION 0004 BC 0007 BE 0007 BE 0010 C 0011 DC			
SVMBOL	SYMBOL	SYMBOL	SYMBOL NP NOA E 1 1 E MTL	SYMBOL	LABEL 2 60 4 10 5 40 6 2 0 6 2 0 6 2 0 8 8 9	PAGE 0007	PACE 0001	:
LOCATION 000078	LJCAT 10N 003390	LOCATION 00000C 000020	LOCATION 000208 00021C 000230	LOS ATTON	LCCATION 0005482 000716 000566 0005CE 001CO			A GS IA
SIZE 0000DC SYMBOL FXC	SIZE DO44ED SYMBOL NST	SIZE DODOZA SYMBOL NR4DIA NR2	SYMBOL K MOSA KNU THICK	SYMBOL	250 250 370 530 610 740 860	•	71321 ***********************************	M. 1089. X95
MATL / HAP : LOCATION OCCO64	PLAT / MAP LDCATION 002260	RAX / MAP COCATION 000008	L DCATION 000204 000216 00022C 000240	LED	LOCATION 0003DA 0005DO 00094E 000C44 000F4 000F8	DATE 71321 001534	CVERLAY AF **********************************	XLOAD, YLOAD, XYLOD, XMON, YMOM, XYMON, YN 1300), 2 (300), 5 LOPA(300), CODE (300), UX (300), UY (300), U
BLOCK / SYMBOL FYT	BLOCK / SYMB3L NC	BLOCK / SYMBOL NR4STF NR1	MAP SYMBOL NLEG J NOOH E33	SUBPROCRAMS CALLED ON SYMBOL	146 146 146 146 146 146 146 146 146 146	1020	LEVEL 4 LOPA) L, MLGO, NL (5), THE TA	0, x4, LD0, x 800), SL D0 1x1, 300), L
COMMON LOCATION 000050 000084	COMMON LOCATION 001130	COMMON LOCATION 000004 000018	SCALAR LUCATION 000200 000214 00022 8	SUBPROC	LOCATION 000392 000694 000662 000666 000676 001176	m	44 PS VERSION 3, LEVEL 4 DATE 71321 **********************************	DMFON / LODA / KLOAD, YLOAD, XYLOD, XMOM, YMOM, YWGM, YGRG, KGSMR, YGSMR,
SYMBOL FXT CMS	SY MBOL NB	SYMBOL NR 4 LOD NR ECLD	SYNBOL NP IA NCS B E I Z	SAMBOL	LABEL 22 0 350 450 560 700 700 700 960 960 960 960 960 960 960 960 960 9	44 PS VERSIO 6100 00152C IEMENTS 0015A0 BYTES 11TY CODE MAS 0	44 PS USRUUTINE L USRUUTINE L OPHON / ONE *XC(11)*HE	COMMON / LODA / COMMON / LODS/ COMMON / LODS/ COMMON = NX*NY + NUMNP = NX*NY + NUMNP = NX*NY + NUMNP = NX*NY +
LOC A TI UN 000000 0000 A0	L2CATION 000000 003380	LOCATFON 000003 000014	LDCATION 0001 FC 0002 10 0002 24	LDC AT 10N 00024C	10CATION 00034 000526 000576 000060 001154	OOL43E WOODL REQUIR HIGHEST SEVER	MODEL C ************************************	9
SYMBOL E SSS	SYMBOL NA NO	SYMBOL NR4 HRECST	SYMBOL NAEVN I NOB E22	SYMBOL	LABEL 210 210 300 420 550 650 650	FORTRAN 1010 TOTAL /	FORTRAN IV 0001 0003 0003	\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$

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	E 71321			
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	21 0,22 0,21 0 21 FACT = 2.0 NA) FACT = 1.0	•	FACT = 2.0 FACT = 1.0 K+N21 = PEVBEFACT		VERSION 3. LEVEL 4	009.0		117-M-21-M17				30,275	'n	-PHYSFACT	3, 290		FACT = 1.0				- 2) 325,335,335	CAL-KRAS I SI KIV			!					331.329.331		JAK - PHYSEACT	¥
250 I = 1.NA	C - 4)	K) - UX	7.5	1. E	Z.	(IUNIT) 70,70,800	3	-	چ ه	300 I = 1.0NA		(L-4) 275,280,275 [T =-].0	(L .EQ. 2)	II .EO. NA) FAC	1-31 290,3 00,290	(1 .60. 1)	E0. 13	(L .EG. 4) L		CONTINUE	(1TYP - 2)	. →	-	- (1-1)*(4*NY-1)		. F	-	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1 - 1 - 0 X	- 2.0	(1 .60. 1) 1 (1+K) = (K(1+	J+K+N1) =
x 78 x	210 74 47 47 47 47 47 47 47 47 47 47 47 47	220 IF 230 FAC	1113		MODEL 44	4	740		ء ء يـ	5 ¥	اب	1F 279 FA	11	# 5	280 IF	290 FA:	- 2	300 15	. 3	320 CC	= 2	165 PR	8.	• 5 • 5	326 H =	_ <u> </u>	327 H	328 CONT	-	FACT	329 FACT	331 UK	330 086
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60 TO 350
BENDING MOMENT ON CYLINDER AND ATRFOIL
                                                                                                                                                                                                                                                                                            345 COM!
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VERSION 3. LEVEL 4 DATE 71321
                                                                                                                                                                                              1F (K - M) 689-688-689
688 FACT = 2-0*PB
689 UY(1-K) = UV(1-K) + COSTHEFACT
UX(1-K) = UV(1-K) + SINTHEFACT
UX(1-K+N1) = UY(1-K+N1) - COSTHEFACT
60 UZ(1-K+N1) = UZ(1-K+N1) - SINTHEFACT
                                                                                                                                                                                                                                                                             00 750 I = 1,4MX
                                                                                                                                                                                                                                                          GENERATE TORSION LOADS DW AIRFOIL
720 NEUD = 5eNY/4 + 1
ARM = 0.0
DO 750 I = 1.8X
                                                                                                                                                                                                                                                                                                                                              747 CONTINUE
DO 750 K = 1,3
FACT = 1,0
T48 FACT = 2,0
749 FACT = 2,0
749 ANS = SLOPA(1)+K)
                                                                                                                                                                                                                                                                                                            GO TO (745,746).L

5 M = 3

C = 2

C = 2

6 M = 1

L = 1
                                                                                                                                                                                                                                                                                                                                                                                                                   Do 760 I = 1, MK
J = (1-1) 0 (4 0 W - 1)
GO TO (755, 756) .L
                                                                                                                                                                               L = 1
CONTINUE
DO 690 K = 1+3
FACT = PA
                                                                                                                                                                                                                                                                                                MODEL 44 PS
                                                                                                                                                                                                                                                                                                                     745
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LOCATION
000010
                                                                                                                                                                                                                                               PAGE 0009
                                                                                                                                                                                                                                                                                                                                                                                                                                                   SYMBOL
                                                                                                                                                                                                                                                                                                                                                                                                              PAGE 0010
                                                                                                                                                                                                                                                                                                                                                                                                                                                   LOCATION
                                                                                                                                                                                                                                                                                                                                                                                                                                          COMMON BLOCK / COMN / MAP SIZE 00006C
LOCATION SYMBOL LOCATION SYMBOL
000004 NLDD 000008 NLIN
000018 NFL 00001C
                                                                          ANG = SLOPALJ+K)

IF (Z(J+K) +LI. 0.0) ANG = ANG - 3.14159

UY(J+K) = UY(J+K) + PAECOS(ANG)0FACT

UZ(J+K) = UZ(J+K) + PAESIN(ANG)0FACT

UZ(J+K) = UZ(J+K) + PAESIN(ANG)0FACT

UX(J+K+N) = UY(J+K+N) - Y(J+K)0UZ(J+K)10FACT

UZ(J+K+N) = UZ(J+K+N) - PAECOS(ANG)0FACT

WALTE (5.5020) I JORKOARN PA
                                                                                                                                                                                                                                              VERSION 3. LEVEL 4 DATE 71321
                                                                                                                                                                                                                                                                                                                                                                                                           VERSION 3. LEVEL 4 CATE 71321
                                                                                                                                                                                                                                                               850 WRITE (3) (UXII), UY (1), UZ(1), I=1, NUMNP) IF (1HOLE .EQ. 0) GD TD 860
                                                                                                                                                    IF (ITYP-2) 810,830,830
                                                     = 1.0
- H) 759.758.759
                                                                                                                                                                                                   820 UZÍNFIX) = 0.0
830 CONTINUE
6020 FDRHAT ( 110,6F15.5 )
                                                                                                                                                                                  S4 CODEINFIX) = 7
                                                                                                                                                        610 NF IX = N2
00 820 I = I
NF IX = NF IX
                                                                                                                                                                                                                                                                                                                                                                                                                                                   SYMBOL
NUMEL
NBN
               GO TO 757
                                                                                                                                         BOO CONTINUE
                                                                                                                                                                                                                                              MODEL 44 PS
                                                                    758 FACT
759 ANG =
                                                                                                                                                                                                                                                                                                                                                                                                              MODEL 44
755
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LOCATION 00002 0 00005C 0000C4	LOCATION 000010	LOCATION	LOCATI ON	LOCATION	0003CC	000360	00000	000410	0000	000428	LOCATION		LOCATION 000484		LCCATION		9000 IE	000038	0000988	000842	982000	000E70	001128	001206	001366	00163	00 16EA	001 9F4
SYMBOL V LOFF XC I TY 5 NNEW	SYMBOL	SYMBOL	SYMBOL	SYMBOL	7.5	ž -	FACT	XX.	COSTH	MFIX	SYMBOL		SV #BOL		LABEL	PAGE 0011	30	9	000	801	120	091	270	320	329	343	348	415
LOCATION 00001C 000030 0000C0 0000C0	LCCATION 00000C 0000020	LOCATION	LOCATION	LOCATION	0003 C8	000300	000000	000418	000440	95 + 000	LOCATION 0004 70		10CATION 0000480		LOCATION	•	90000	00091E	000996	000B 2E	04000	00000E	001056	₩ Z 100	0013BE	001576	W69100	001656 00199A
SIZE ODODES SYMBOL THETA TC IOPT ICNT	SIZE 000024 Symbol Xmon Yosmr	SIZE 0012CO SYMBOL UZ	SIZE 000004 SYMBOL	SYMBOL	¥	YEST	æ	Ě.	SINTH	TORK	SLOPA		SIN		LABEL		25	8	C E	101	110	170	250	300	328 336	341	347	410
ONE / MAP SI LOCATION 000008 00002C 00000C 00000C	LODA / MAP SI LOCATION 000008 00001C	LODS / MAP ST LOCATION CO0960	DUMP / MAP SI LOCATION	LOCATION	000364	000 308 0003 FC	0004000	\$1,000	0000	000420	LOCATION 00046C	160	L OCATION 00047C	3	LOCATION	DATE 71321	00079E	00091E	484000	\$18000	000BCA	260000	00106E	001210	001344	001576	001686	001652
SYMBOL SYMBOL THK DUM NY 1TET	SYMBOL XYLOD XQSHR	SYMBOL UY	SYMBOL SYMBOL	SYMBOL	N2	XF 1X	×	PXYB		A NG	SYMBOL 2	SUBPROGRAMS CALLED	SYMBOL AT AN2	HAP	LABEL	LEVEL 4	90	~ :	2 4	106	109	8 2	230	2 90	335	340	346	400
COMNON LOCATION 000004 000028 000000	COMMON LOCATION 000004 000018	COMMON LOCATION 000480	CONMON LOCATION	SCALAR	000300	0003.68	0003FC	01000	96 1000	00044000	ARRAY HAP LOCATION S 000468 Z	SUBPROC	000478	1	LCATION	VERSION 3. LI	209000	OCCORE	4 4 6 C C C C	000ABA	000BBC	\$2000 \$2000	00105A	001508	001390	00156A	99100	+88 100
SYMBOL HT THT MM 1HOLE	SYMBOL YLO AD Toro	SYMBOL	SYMB OL	SYMBOL	- F	LUNIT	_	PXVA	PA	MARM	SYMBOL		SORT		L A B EL	r	20	35	٠ «	105	112	33	220	280	326 330	339	345	3 60
LOCATION 000000 000024 000088 000008	LCC A T ION 000000 000014	LOCAT 10N 000000	LOCATION	LOCATION	0003.00	000364	0003F@	200400	96,000	000448	LOCATION 000444		000474		LOCATION	14 MODEL 46	000 5A8	0008BE	0.000	000480	000B6A	01000	000100	001 1AE	0012EE	245100	259100	268 100
SYMBOL SIDE YZDFF HED IFACE	SYMBOL XLOAD XYMOH	SYMBOL	SYMBOL ITEST	SYMBOL		KONST	¥	<u> </u>	RAD2	Q E	SYMBOL		10COM		LABEL	FORTRAN	0	32	2 6	103	111	061	210	275	325	330	346	300

001806 001060 001162 002162 002360 002730			100.47104	 	
844466004 848466004	1000	2000	SVABOL	1000	4
00188C 001 CE5 001 F52 002 84C 002 420 002 84Z 002 84Z 002 84Z	4 PAGE	PS44 1020	LOCATI ON	PAGE STREED TO	
505 520 520 520 620 620 755 620 755 620 755 620 755 620 620 755 620 755 620 755 755 755 755 755 755 755 755 755 75			20 87	71321 ***********************************	ST(3,10.4).INTL NRECLO.MI.OMZ .MK(6) .MK(6) .MK(1) .MK(1) .MK(1) .MK(1) .MK(1) .MK(1) .MK(1)
001 AC 2 001 C 3 4 001 C 3 4 002 C C 002 S C C 002 S T C 002 S T C 002 S T C	DATE 71321	OATE 71321	L OCATION 000000	E E E	1 . CODE 6001 ID 1001600 ELM CODE 6001 ID 1000 ID 10
9000 9000 9000 9000 9000 9000	LEVEL 4 OVERLAY 8 **	IEVEL 4	SUBPROGRAMS CALLED SYMBOL STIFF	LEVEL 4 D DOSSE OVERLA EL, NLOD, NLIN (15), THETA, MANALITA, MA	M (6), T. ETR. E TF. MADDIA, TO. 1000, 1
001A80 001C12 001E8C 002E170 00228C 0025%E 0025%E	5	STRESS STIFFEES MATRIK LOAD "TETRALO") STIFF III N PS VERSION 3.	SUBPRIOR OCCODA.	**************************************	CDE(600) 2D0(600) 1EUM CDE(600) 2D0(600) 1EUM CONTRAK, MARA, MARLOD, MRAST NSION DUMICOOD, DUMZ(60) NSION DUMICOOD, DUMZ(60) NSION DUMICOOD, DUMZ(60) NSION DUMICOOD, DUMZ(60) NALENCE (H(13), JR), (M(12) ND 3
450 515 515 610 640 748 748 757 610 650		* 644	SYMBOL LOCATES STRESS GOODE REMEMTS GOOL OF BYTES RITY CODE WAS 0	SUBROUTINE STRESS COMMON / COMM / W COMMON / ONE / SI	COMMON / ELM / SI COMMON / ELM / SI COMMON / ELM / SI COMMON SIGN DUMICED DIME NS IGN DUMICED DIME NS IGN DUMICED OF SIGN DUMICED FOULVALENCE (ML(3) FOULVALENCE (ML(3)
001408 0018EA 0018EA 0018Z0 0021DZ 0021DZ 00239A 00239A 00235A 00235A 00235A	FORTRAN	1300M	LOCATION 0000DO NEMDRY REQUIRENE HIGHEST SEVERITY EXEC FORTRAN (0CO	MODEL C	v
420 5120 7444 7444 7550 7500 7500 7500 7500 750	OPPILIT MICHASTASS ENEC FONTRAN IV 0001	0003 0005 0006 0007 0007	SYMBOL LOAD TOTAL TOTAL	FOR TRAN 1V 00001 00003 00003	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

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                                                                                                                                                                                                                                                                                                                                                                     TAPE
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                           READ AND PRINT OF ELEMENT PROPERTIES
BEGIN BOUX-KEEPING ON ADDITIONAL MODES AT MID-POINTS OF SIDES
                                                                                                                                                                                                                                                                                                                                                                                                           DO 110 N=1,NUMNP

85 MRITE(6.6020)

6020 FORMAT (641N0DAL, /, 644 POINT, 4x, 14K, 8x, 14Y, 8x, 14Z,//)

90 PPRINT=50

90 PPRINT=10

95 MRITE(6.6030110(N), X(N),Y(N),Z(N)

4030 FORMAT (14, 4F10.4,3F15.4)

110 CONTINJE
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                        DG 300 1=1, WUNEL
READ (3) IDP, IP, IQ, IR, IS, ETR, ELGT, EPRN, G, T, INTL
JQ = 10
JQ = 10
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                                                                                                                010 FORMAT (INT ///
1 30H- NUMBER OF NODAL POINTS----- I4 /
2 30HO NUMBER OF ELEMENTS------ I4 /
3 30HO NUMBER OF LOAD CASES ------- I4 )
READ NODAL DATA FROM TRANSFER TAPE
READ AND PRINT OF NODAL POINT OATA
50 READ (3) (10(11, X(11), V(1), Z(11), I=1, NUMMP)
IF (1CNT-1) 80,80,120
                                                                             NUMMP, NUMEL, N. DD
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                       DO 250 M = 1.MADD
IF (KAS-INI(NADD+1-M1)) 250,260,250
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          .Eq. 21 GO TO 240
                                              IF (ICNT-1) 45,45,50
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 210 N(11) - NID(10,1R)
IN(1) - HAXO(10,1R)
GO TO 210
215 N(11) - NID(10,1S)
IN(1) - NAXO(10,1S)
216 JP-N(1)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                      NI(2)*NID(IR, IP)
IN(2)*MAXO(IR, IP)
JQ * NI(2)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               MODEL 44 PS
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                         120 KA1-0
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                      245 KAS
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6.T.1M1	ADDITIONAL NODES USING TAPE 3 IT IONAL NODES	PAGE 0003		
240 CONTINUE ALLL = RAS ALLL = RAS ALLL = RAS 270 CONTINUE 290 CONTINUE 290 CONTINUE 290 CONTINUE 290 CONTINUE 390 CONTINUE 300 CONTINUE 300 CONTINUE 300 CONTINUE 300 CONTINUE 300 CONTINUE	READ (3) (UN(I), UV(I), UZ(I), I-1, NUMMP) READ (3) (UN(I), UV(I), UZ(I), I-1, NUMMP) READ (3) (CODE(I), I-1, NUMMP) RE-CR DER LOAD MATRIX TO ACCOUNT FOR ADDITIONAL NO C MODITY BOUNDARY VALUES TO INCLUDE ADDITIONAL NODES DUMI(I) = 0.0 DUMI(I) = 0.0	MODEL 44 PS VERSION 3, LEVEL 4 DATE 71321 DUMBLII - 0.0 1.0111-10111 DUMBLII - UMIII DUMBLII - UMIII DUMBLII - 12111 DUMBLII - 22111	DO 400 I = 1.NUMMP DO 370 J = 1.NUMMP DO 370 J = 1.NUMMP IF (ID(I)-IN(J)) 370,340,370 360 I = IDC(KA7) = NI(J) IO2 = IDC(KA7) = ID(+1000 IF (UK(ID2)) 364,365,362 364 IF (UK(ID2)) 364,365,364 364 IF (UK(ID2)) 364,365,364	ABSTURE AFINICA TO TO TO TO TO TO TO TO TO TO TO TO TO T
0066 0066 0066 0066 0068 0070 0072	00015	0082 0083 0083 0083 0085 0085	P	010000000000000000000000000000000000000

	PA CE 0001			b./.																TAPE 2			
	LEVEL 4 DATE 71321			ASE, 14.7, SK, SHOEM, ZK, SHOLD, /.						0.20			0							10P.KP.KQ.KR.KS.JP.JQ.ETR.ELGT.EPRN.G.T. 1NTL			
IF II -NUMMP 380,390,380 NA7 = KA7 + 1 DOKKA7 = 10[1+1] DUMI(KA7) = UXII+1 DUMI(KA7) = UXII+1 DUMIKA7 = UXII+1 DUMIKA7 = CODE[1+1 CONTINUE CONTINUE DUMIKA7 = 1,400ES	44 PS VERSION 3.	#13-21- DUM1(1)	7	10.	CONTINUE MPRINT-MPRINT-1 MRITER - 60701 1-100619 - 864-29 - 864-39 - 8643	FORM		CODE (11)	DO 480 I - 1, NODES	22	DO 470 J = 1.NJRNP IF (ID (1)-KA1) 440-430-443	200	2.	CONTINU	C001 - DURICK1)	: £		DO 720 I = 1. MINEL				:3	# 11 11 11 11 11 11 11 11 11 11 11 11 11
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FORTRAY IV	MODEL 44 P5 VERSION 3, LEVEL 4 DATE 71321 GO TO 550 SID IF EXC-IDEALL STACKSON 510
0181 0181 0182 0183	- 6 T
	545 IF (KS-10(J)) 550,547,550 547 IS - J 1ELM - 2 550 CONTINUE
0189	560 CONTINUE 00 670 J = 1,400ES 1F (F-TOCLI) 560,570,580
25610	8223
010000000000000000000000000000000000000	22.
0203 0204 0204	60 TO 660 IF LD-100(J) I LT J 60 TO 660
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0212 0213 0214 0215 0216	ここといここ
0210 0220 0221 0221 0222 0222	IF (NCIN IF (ICUT IF (MPRIN MRITE(6, FORMAT(0) I 2x, MJ, I 1, 196, MPRINT=5
FORTRAN IV	NODEL 44 PS VERSION 3, LEVEL 4 DATE 71321 PAGE 0004
0225 0224 0227 0228	WRITE (6.6050) IDP, IP, IQ, IR, IS, ELGT, EPRM, G, T, LP, LQ, LR, LS, LT, LU 6050 FORMAT (16.414 , 4F12.0, F10.4.614) 697 CONTINUE C STORE ELEMENTAL DATA ON TAPE 1 WRITE(1.WR) IDP, KP, KQ, KR, KS, LM, WM, ETA, ELGT, EPRM, G, T, IMTL, IELM TAPE 1 COMPUTE BAND WIDTH
0230 0231 0232	IF (LU .EQ. 0) LU = LT NMAX = MAXO(LF.LQ.LR.LS.LT.LU) NMIN = MINO! P.LQ.LR.LS.LT.LU)

		LOCATION 000010 LOCATION 00002 000005 000006 000006 000006 000000 LOCATION 00051 00051 00051 00051
		SYMBOL HBAND SYMBOL YLOFF XC 177 HNEM SYMBOL IELM IR ELGT
AND 2" RIX W FILE		LOCATION 00000C 00001C 00001C 00001C 00001C 00001C 00001C 00001C 00001C 00001C 00001C 00001C 00001C 00001C 00001C
AND 2 BY FILE 1 BY FILE 4 ROWS IN MAT	W FILE ♦?	SIZE 00006C SYNBOL NLIN SIZE 0000EB SYNBOL THETA THETA THETA TO IOPT IOPT IOPT IOPT IOPT IOPT IOPT IOPT SYNBOL IOC SYNBOL IOC SYNBOL IOC SYNBOL IOC IOC IOC IOC IOC IOC IOC IOC
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K-INMAX-NMIN) 700,700,710 NUE NUE NUE IS THE NUMBER OF RECORDS REQUIRE NUEL (6.1) NOR1 1(1H ,15.° IS THE NUMBER OF RECO =30(KK+1) 1S THE NUMBER OF BYTES PER RECO =30(KK+1) 1S THE NUMBER OF BYTES PER RECO =30(KK+1) 1S THE NUMBER OF BYTES PER RECO =400 NUEL 11H ,16.° IS THE NUMBER OF BYTE = HBAND/3 CNT ,60. I) MRITE (6,6100) NBAND T (1 M1 , 5%, 1541/2 BANDMIOTH 9 ONCEVIRATED FORCES	THE STATE OF THE S	COMMON 1000004 000004 000004 000004 000008 000008 000008 000008 000008 000008 000008 000008 000008 000008 00008 00008
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SYMBOL	SV MBOL LS	SYNODL KA1 KA3 KA3 MRS MI MI MI MHAX	SYMBOL	z z	SYMBOL	LABEL 85 110 218 260 300	# 6 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	865 865 865 865 865 865 865 865 865 865	
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BLDCK / SYMBOL NR4STF NR1	EQUIVALENCE DATA ON SYMBOL LQ	AR MAP SYRBOL MPRINT M IDI KA2 COD2 KS WOBYT	MAP SYMBOL EVEL 4		SUBPROGRAMS CALLED ON SYMBOL OUTIN	MAP LABEL 50 95 210 245 245 362	968 360 6070 417 450	600 650 675 700 165	
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SYMBOL NE4LOD NRECLD	LU LP SYMBOL	SYMBOL I ADD NADD KA7 J CLOD- COD1 KR	SYMBOL 44 PS	20	SYMBOL	LABEL 6010 90 200 240 240 360	964 940 940 940 940 940	541 590 640 670 693 2	
LOCATION 000000 000014	LOCATION 0002C0 0002D0 0002E0	LOCATION 0002 E4 0002 F8 0003 F8 0003 20 0003 48 0003 48	V MODEL	000380	LOCATION 003710 003724	LOCATION 603854 603950 003A7C 003BD0 003CE 003CE	004068 0041 A4 0042EC 00441 6 0044EE	0046F2 004772 004772 00483 004864 004870	HEHORY REQUIRENENTS
SYMBOL MR4 MRECST	SYMBOL HH LT JR	SVRBOL NTAPE 10P KAS KAS KAS KAS KAS KAS	SYMBOL FORTRAN I		SYFECT. 19COMB WING	LABEL 45 6020 120 220 270 350	8 4 4 4 4 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6		TOTAL ME

COMPILER HIGHEST SEVERITY CODE WAS O 1/STIFF ENEC FORTANIACO. NAP)

	PSTF0010								•																												PAGE 0002									
**************************************	NUTTER WHELE ALOD MIN HBAND NUMBER NBW MEL(20)	1300 141 300 1401 6001 01 6001 01 6001	(18,10), LM(6), T, ETR, EPRM, ELGT, 6, ST(3, 10, 4), 1MT, ''''' A4LOD, WASSTF, MADEC, WRECST, WRECLD, MADEL MADE		(2700).DIAG(1800)		3471												ONEONNO NO CHONON CONTROL OF THE TANKE TO TH														.191560.11)	CO TO 557			3, LEVEL 4 DATE 71321	1	TE HOLD ST			110P,ST,KP,KQ,KK,KS,LM,ETR,E.GT,BPR,6,T,INTL,MY,WOO TAPE				
SUS ROUTING STIFF	COMMON / COMN / NUTTO	E16001,10	/ ELM / S Rax/mr4.n	/DUMP / N	200	TELLIAL VALUE	REMIND 3	NR 4 = NR 4 STF	NR S=3+NUMNP	MEMBAND-1	AC 244	A(1)=0.	DO 50 1-1.NRS	50 DIAGILI=0.0	DO SI CATANOMAN		DO 1000 N=1 NUMEL	2	TO THE TOTAL	H .EQ. 23 GO	II - MM(3)	MACA) - KM(2)	11 = (2) H	TREST TREES		NNOD = S	50 CO 63	60 10					11H .7110/ 11H	ST . EQ. 03		00 556 1 - 1,0003	MODEL 44 PS VERSION 3	20.05	SSO WRITE (6.6000) (SII.63). LELWOUS!	NO GORMAT (10612-0)	-	WRITE (3	1004 00		00 620 I=1,NNOD	HE ?
1000	0005		\$000	9000	2000		6000	0100	1100	2100	100	\$100	9 100	1100	9000	0050	1200	2005	4200	9 200	9200	0027	8700	0030	1600	200	6000	5600	90036	20037	00 30	•	•	20042	*****	8900	FORTRAN IV HO		9500		•	•		!	0053	9500

				PAGE 0003	
	0 004)+S(KK,LL) []+S(KK,KK)	INUE INUE INUE IESS.EG.O) GO TO 1011 AT (244 MERGED STIFFMESS MATRIX) NRASTF OLO I=1.MRS.3 OLOTIN [2.A.NE]	ATIIH .4HROWS.IS.3X.10E12.4/11M .11E12.4)) INUE INUE RT B V .S AND SET UP DIAG TAPE 000 I=1.NURNP 00E(I)99912001,2002,2002 (I)=CODE(I)+0.00001 00E(I) 01.2.3.4.5.4.7).MC	M 3, LEVEL 4 DATE 71321	
LR4=NR4 CALL CUTIN (2.A.ME) NR4=LR4 DG 700 K=1.3 NS=MP (K-1) II=LNI1)+K KK=301-3+K DG 800 J=1,NNDD 803 DG 805 L= 1.3	JJ=[M[J]+[15-1.56=11) GO T 11-1.56=11) GO T 11-1.51-11-10-10-10 A(JJ) -A(JJ) CONTINUE CON	700 CONTINUE CALL DUTIN (1,4,NE) B20 CONTINUE 1000 CONTINUE 1F(NTEST.EQ.O) GD TO 1011 WRITE(6,1005) 1005 FORMAT(24M MERGED STIFFME NR4=NR4STF DO 1010 [=1,NR5,3] CALL DUTIN (2,4,NE)	1006 FORMATIH "4HRDM6.15.3X.10E12. 1010 CONTINUE INSERT B V .S. AND SET UP DIAG DO 2000 1=1.NURNP IF(CODE(1)99912001.2002.2002 2002 CODE(1)-CODE(1)+0.00001 NG=CODE(1) GO TO (1.2.3.4.5.4.7).NC I NS=1	MODEL 44 PS VERSION 2 GOTO 8 2 NS=2 NE=2 GOTO 8 3 NS=3 NE=3	S NS=1 S NS=1 S NS=1 S NS=1 S NS=2 S NS=2 S NS=2 S NS=2 S NS=2 S NS=2 S NS=2 S NS=3 S
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			11 181 13	1			•		1		
				LOCATION 000010	LOCATION 001770	LOCATION 000530	L OCATION 000010	LOCATION	LOCATION 0001 C0 0001 D4 0001 E5 0001 FC	LOCATION	LOCATION
			PAGE 0004	SYMBOL	SYMBOL	SYMBOL	SYMBOL	SYMBOL	SYMBOL J J K K K K K K K K K K K K K K K K K	SVABOL	SY MOOL
	: 	1476	•	LOCAT10M 00000C	LOCATION 000E19 003CF0	LCCATION 00052C 00089C	LOCATION 000000 0000020	LOCATION	LOCATION 00018C 0001D0 0001F4 000) F8	LOCATION	LOCATION
				SIZE 00006C SYMBOL NLIN	SIZE DO3CF4 SYMBOL UX IEL4	SIZE OGOBAG Symbol ETR INTL	SIZE DODDZ4 SYMBOL NR 4DI A NR 2	S12E 000004 SYMBOL	SYMBOL I RQ MNOD II	SYMBOL	SYMBOL
•12 •1.0 E•12	DIAG(1),[-1,MRS) IM ,11E12.4)	•	DATE 71321	COMN / PAP 1 LUCATION 000008	XVZ / HAP LOCATION - 000960 003390	ELN / MAP : COOS 28 OOO 53C	LOCATION COCOCS COCOCS	DUR / NAP :	LDCATION 0001180 0001 CC 0001E0 0001F4	LOCATION 004864	LED LOCATION 034884
3+K)+L.0E	_	•	LEVEL 4	BLJCK / SYMBOL NLOC NEL	SYMBOL SYMBOL 2 100	BLOCK / SYMBOL T ST	BLCCK / SYMBOL NR4STF NR1	BLOCK / SYMBOL	NAP SYNBOL NE NS NS CL	MAP SYMBOL MM	SUBPROGRAMS CALLED ON SYMBOL TRIMS
NUE • NS,NE,NJ • (301–34K)•DIAG(301–34K)•L.OE+12 UE UE		(150(1), I-1, NUMP)	VERSION 3.	COMMON LDCAT I ON 000004 0000 18	COMMON LOCATION - 000480 002A30	COMMON L OCATION 000510 000538	COMMON L OCATION 000004 000018	COMMON LOCATION	SCALAR LOCATION 000184 00016 000110 000110	ARRAY LOCATEON OO2C44	SUBPRO LOCATION 004880
CONTINUE DO 9 K= NS- DIAGI3-E CONTINUE CONTINUE	NR4=NR4DIA CALL OUTIN (1) IFINTEST.EQ.O FORMATIC	3B_	\$ \$	SYMBOL NUMEL NON	SYMBOL Y CODE	SYMBOL	SYMBOL NR4LOD NRECLD	SYMBOL	SYNBOL F 10P 11 L	SYMBOL	SYMBOL OUT IN
2001	1019		IV NODEL	LOCATION 000000 000014	LOCATION 000000 002 000	LOCATION 000000 000534	LOCATION 000000 000014	LOCATION 000000	LOCATION 000180 0001C4 0001B6 0001EC	LOCATION 000214	LOCATION 00487C
0113 0114 0115 0116	0118 0120 0121 0122	0124	FORTRAN	SYMBOL	Zn X Toews	SYMBOL S S ELGT	SYMBOL MARA	SYMBOL NTEST	SYBOL HRS RS LR4	SYMBOL	SYMBOL

LOCATION 005-80C 005-626 004-666 005-72C	005200		LOCATION	LOCATION		
LABEL 6000 804 1009	PAGE 0005 2000	PAGE 0001	PAGE DODZ	SYMBOL SYMBOL	PAGE 0001	
LOCATION 004AFA 004C 26 004E2A 004EEB 005058	005148 005206		LOCATION	LOCATION		MTL 2(3).
LABEL 62 557 805 1000	2001			SYMBOL		57(3,18,4).IMTL 57(3,18,4).IMTL 61(3).AL2(3)
LOCATION 0049CE 0049CE 0040CE 004ED2 009ED2	DATE 71321 00512E 0051AD	DATE 71321	DATE 71321 1 MAP LOCATION	LOCATION LOCATION	DATE 71321	UTINE TRIM6 N / XYZ / X(30C),V(30O),Z(30O),UX(60O),UV(60O),UZ(60O) ODE(60O),IDD(60O),IELM N / ELM / SILO,BO,LM(6),TETR,EPRM,ELGT,G,ST(3,10,4),IMTL N / ELM / SILO,BO,LM(6),TETR,EPRM,ELGT,G,ST(3,10,4),IMTL T(6,6),AST(10,10),ETA(3),ALAM(12,18), SION PHYX(3,6),PHYY(3,6),ALAM(12,18), SION STA(10,10),STR(3,10,4),STRA(3,12,4),IM(6), ITIALIZE STIFFNESS MATRIX O J = 1,14 O J = 1,14 S J = 1,14 S J = 1,3
EL MAPEL LABEL 51 556 903 920 903 920 1011	LEVEL 4	· LEVEL 4	N 3. LEVEL 4 : EQUIVALENCE DATA ON SYNBOL	SCALAN MAP ON SYMBOL SUBPROGRANS CALLED ON SYMBOL	LEVEL 4	- 9 - E
LABEL LOCATION 00498A 00483E 004CEC 004E62	VERSION 3. 005114 005198 005286	8 %	VERSION 3. EQUI	SCALAR LDCATION 0000DC SUBPRO LDCATION 0000E4	EO BYTES S O VERSION 3.	F SE E E E E E E
LABEL 50 1007 400 700 1010	0050FA 3 005114 005184 8 005198 005284 1020 005286 HEMORY REQUIREMENTS 005364 BYTES	(18CO, NAP) (18CO, NAP) (14 PS VERS) (14 PS VERS) (10 = MAXO(KP, KQ) PRETURN (10 = MAXO(KP, KQ) PRETURN (10 = MAXO(KP, KQ) PRETURN	SV BOL	SYMBOL KQ SYMBOL MINO	REMENTS ODDIED RITY CODE MAS HOCO, MAP)	SUBACUTINE T COMMON / XVZ 1 CODE(600 COMMENSION P 1 OMA (6.6). DIMENSION ST INTITALIZ DI 450 J I DO 450 J I DO 450 J I DO 475 J I
LOCATION 00495C 004534 004C34 004E66	0050FA 005184 005184 0052A4	EXEC FORTRANIBCO EXEC FORTRANIBCO N IV MODEL 44 FUNC N IV MODEL 44 END	IV MODEL	LOCATION 000008 LOCATION 0000E0	MEMORY REGUIREMEN HIGHEST SEVERITY EXEC FORTRANICO.	. c
1.48£L 4.9 65 30009 860	FORTRAN 2 2 1 1019 TO 1	COMPLEA HIGH	FORTRAN SYMBOL NID	SYMBOL KP SYMBOL MAKS	TOTAL NEW COMPILEY HIG //TRIMG EKE FORTMAN IV	000000000000000000000000000000000000000

		PAGE 0003	
J=1.NO33 I=1.3 I=1.3 KT)= AL2 NYT)= AL2	AS = KRPALZII;+YRPALZIZ)>RRPALZIS) BS = KRPALZII;+YRPALZIZ)>RPALZIS) BS = KRPALZII;+YRPALZIZ)>RPALZIS) BS = KRPALZII;+YRPALZIZ) BS = KRPALZIZ = RPAT	PHYK(1,2) = -820AI PHYK(2,2) = -3.00PHYK(1,2) PHYK(1,3) = -180AI 44 PS VERSION 3, LEVEL 4 DATE 71321 PHYK(2,3) = -3.00PHYK(1,3) PHYK(3,4) = -4.00 PHYK(1,3) PHYK(3,4) = -4.00 PHYK(1,2) PHYK(3,4) = -4.00 PHYK(2,1) PHYK(3,4) = 0.0	MYY(16.5) = PYY(11.3) PHYY(13.4) = -3. OPPHYY(1.3) PHYY(13.4) = -4. OPPHYY(1.3) PHYY(13.4) = -0.0 PHYY(13.4) = 0.0 PHYY(13.5) = -4. OPPHY(1.3) PHYY(13.5) = PHYY(13.5) PHYY(13.5) = PHYY(13.5) PHYY(13.6) = PHYY(2.5) PHYY(13.6) = PHYY(13.6) PHYY(13.6) = PHYY(13.6) PHYY(13.6) PH
DD 222 NT=KT+ ALAMCJ- ALAMCJ- ALAMCJ- TI TI TI TI TI TI TI TI TI TI TI TI TI T	TARE NEPER	1 3 SE	

	PAGE 0004	MGE ROWS & COUS		
201 QMAT(II.I)= 2.00QMAT(II.I) CALL TRPRD (PHYX.DMAT.PHYX.DUM.ETR .3.6) DO 202 I=1.6 DO 202 J=1.6 202 S(I.J) = 8(I.J) + DUM(I.J) CALL TRPRD (PHYY.QMAT.PHYY.DUM.G .3.6) DO 205 I=1.6 DO 205 I=1.6 DO 205 I=1.6 DO 205 I=1.6 DO 206 I=1.6 DO 206 I=1.6 DO 206 I=1.6 DO 206 I=1.6 DO 206 I=1.6 DO 206 I=1.6 DO 206 I=1.6 DO 206 I=1.6 DO 206 I=1.6 DO 206 I=1.6 DO 206 I=1.6	MODEL 44 PS VERSION 3, LEVEL 4 DATE 71321 DO 211 1=1.6 DO 211 J=1.6 221 S(1,J+6) = S(1,J+6) + DUM(1,J) CALL TRPRO (PMY,QMAT,PMY,DUM,ELGT,3,6) DO 214 1=1.6 DO 214 J=1.6	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	AST(10.1) = S(1.1) AST(2.1) = S(2.1) AST(4.1) = S(2.1) AST(6.1) = S(4.1) AST(6.1) = S(4.1) AST(8.1) = S(4.1) AST(10.1) = S(4.1) AST(10.1) = S(6.1) AST(10.1) = S(10.1) AST(10.1) = S(10.1)	DO 220 1=1,12 S(1,1) = AST(1,1) S(1,2) = AST(1,1) S(1,3) = AST(1,2) S(1,3) = AST(1,2) S(1,5) = AST(1,2) S(1,5) = AST(1,3) S(1,5) = AST(1,3) S(1,5) = AST(1,1) S(1,7) = AST(1,1) S(1,8) = AST(1,1) S(1,8) = AST(1,1) S(1,1) = AST(1,1) S(1,1) = AST(1,1) S(1,1) = AST(1,1) S(1,1) = AST(1,1) S(1,1) = AST(1,1) C(1,1) = AST(1,1) S(1,1) = AST(1,1) C(1,1) = AST(1,1)
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; ; ;	71321						i											:															
ET	3. LEVEL 4 DATE	H*(-1.)		+ AST(1,K) + S(K+10,J)			S(1,K+10)*AK21(K+J)		•	SINESS MATRICES	250	m r	•	- T T.	305	,306),L 04,303 } . L		•			•			1)-1.1081041	2)-1.)*82*A1	1081+2ETA(1	11eB3+7E14(3)eB1le4(1)	77	- CETA(W)-IL. DAM WAR I	LETA(3) = A2 + ZETA(2) = A3 -A3			N IN IN
1 230,240,230 1 = \$(12,12)/DET 1 = \$(11,111/DET	VERSION 3,	= S(11,12)/DET#(-1.	1,10	- 1.2	=1,10	0.0	+ 17 +	0141-	2F1C.0	UP APPROPRIATE	M.EQ. 1) 60 TO	.EQ.1) 1	- 11K, 1KL	*1+3 0-0	. EQ.2) G	308 304 307	1.0	1.0	5 i.0		1.0/3.0	1,1	1,10) = 0.0) = (4.02ETAL)) = (4.02ETA)) = 4.0(ZETA	1 - 4 - (ZETA		*	L)= 4.0(26TA(3	9.1	ST (2,1)	al, 3
1F (DET) AST(1,1) AST(2,2)	44 PS	AST(1,2)	00 501 J	N 502 K	00 503 0	1ST (1, 1)	(L,1) TS	20 204	DRMAT	EK : 1	IKL = 4 IF (IELP	IF CAKE	300 00	00 301 1	IF (JKL	0 TO (3	ETA11)=	ETA131	50 TO 30	20 10 30	ETA(1)=	SONT INCE	310	57(1, J.L 57(1, 1, L	1111.2.1	T(1,4,6	ST(1,6,L	T (2, 8, L	1112.9.1	ST(2.11.	: 2 :		14 0
230	IV MODEL			7 106		~ h	503	4	200	240 1			250 0	301 2			303	304 2	307 2		308	305		310 5	-	, .	., 5,	-1 47		· W •	7 Q 4	410 \$	ه د
0162	FORTRAN I	0185	0188	0190	0192 Cl 93	4610	9610	9610	05 00	1020	0202	0204	0200	0200	6020	0210	0212	0214	0215	0217	0219	02.20	0222	0223	0225	0227	0229	0230	0232	0234	9536	0236	9820

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L.K4+H) = STA(K3+L,K4+H) + S(201-2+L,201-2+H)
VERSION 3, LEVEL 4
                                                                                                                                                                                                                    .51. 3) GO TO 700
                                                                                                                                                           K3 = IKIL

D0 480 L

D0 490 L

STR (1, K3+

D0 490 L

K3 = IK(1)

K4 = IK(1)

D0 490 L

D0 490 L

STA(K3+

CONTINUE
                                                                                                                                                                                                                                                   6200
                                                                                                                                                FORTRAN IV
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IF (ABSIDET) .LT. 1.0 E-10) DET = 1.0 QMAT(11.1) = STA(14.14)/DET QMAT(2.2) = STA(13.13.4)/DET QMAT(12.2) = STA(13.13.4)/DET QMAT(12.2) = STA(13.13.4)/DET QMAT(12.2) = QMAT(1.2) DO 510 1 = 1.2 ANZIGO J = 1.2 ANZIGO J = 1.2 ANZIGO J = 1.2 ANZIGO J = 1.2 ANZIGO J = 1.2 STA(11.3) = ANZIGO J = 1.2	DO 520 1 = 1,12 AST(1,J) = 0.0 DO 520 J = 1,12 DO 520 IJ = 1,2 DO 530 I = 1,12 CO 530 I = 1,12 CO 550 K = 1,4 DO 550 K = 1,4 DO 550 K = 1,4 DO 550 K = 1,4 STRA(1,J) = 85RA(1,J) = 85 STRA(1,J) = 85RA(1,J) = 85 STRA(1,J) = 87RA(1,J) = 0.0	15.1 1. 15.1 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	DO 555 J = 1,12 555 ST(1,1,K) = ST(1,1,K)/2.0 DO 556 I = 1,12 556 WRITE (6,650) (ST(1,1),J=1,12) C GENERAL FORD (ALMYS, ALMYS, LMS, AL	DO 417 1=15 ST(1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-	
02.99 02.99 03.00 03.01 03.04 03.04 03.05 03.05	0309 0309 0310 0312 0312 0314 0316 0317	0320 0321 0322 0323 0324 0328 0328	0327 0320 0330 0331 0332 0333	0335 033987 03348 0344 0344 0344 0344	0346 0347 0350 0351 0353

				LOCATION 001770	LOCATION 000530	LCCATION 00033 00034C 000360 000374 00038C 00038C	LOCATION 0007CC 000ESC
		9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9		SYMBOL LY	SYMBOL	SYMBOL JKL VKG VKG K Z HOO3 A 3	SYMBOL AL2 DUM
				LOCATION 000E1 0 003CF 0	LDCATION 00052 C 00089C	LDCATION 000334 000334 000337 000337 000384 000384 000360	LOCATION 0007C0 000084 00160C
į				SIZE 003CF4 SYMBOL UX IELM	SIZE OCOBAO SYMBOL ETR INTL	5 Y MG CL 1 J K 1 J K 2 K P 2 K T 8 K T 8 C T 8 C T	SYMBOL AL AK21 IK
	AMIKAJI	DATE 71321	3	(YZ / MAP SIZE LOCATION 000960 003390	ELM / MAP LOCATION 000528 00053C	LUCAT I UN 000330 000344 000346 000360 000380 000394	LOCATION 000460 000078 00149C
i i	+ PHYXII.K) MLAHIK, J.	LEVEL 4 ; DUM, W, 3, 4	IK, I)•DUM(COMMON BLOCK / XYZ ON SYMBOL Z TDO	BLOCK / SYMBOL T ST	SYMBOL SYMBOL N TR VRP VRP VRP VRP VRP VRP VRP VRP VRP VR	MAP SYMBOL ALAM ZETA STRA
	1.9 1.9 1.6 1.6 1.6 1.6 1.6 1.6 1.6 1.6 1.6 1.6	VERSION 3. 2. 2. PHYX.QMAT.PHYX 1.6	AST(1.3) + ALAM(K, 1)*DUM(K,3) = 1.9 0.0 = 1.6 S(1.3) + AST(1.K)*ALAM(K,3)	COMMON LOCATION 000480 002 A30	CONMON LDCATION 000510 000538	SCALAR 00032 000340 000340 00036 000370 00034 000386	ARRAY MAP LCCATION S 000418 A 000868 Z 0011FC S
PHYK (3, 4) # PHYK (3, 4) # PHYK (3, 4) # PHYK (3, 6) # DO 760 # Quart (1, 1) # PHYK (1, 1) # PHYK (1, 1) # CONTINUE		"AA" "	AST(15.) = AST(15.) =	SYMBOL Y Y CODE	SYMBOL	SYMBOL J J J Z Z Z Z Z Z B J M A RE A Z	SYMBOL PHYV AST STR
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000000000000000000000000000000000000000	3. 52	6 0375 0375 0375 0376 0376	0382	SYMBOL	SYMBOL S ELGT	SYMBOL 11 12 20P 20P 72 72 83 1KL 13	SYMBOL PHYX DHAT STA

LOCATION	LCCATION 0019EC 0019EC 0019BE 0019BE 0029A 0023A6 002AA2 002AA2 002AA2 002AA2	00 340 5		1.0C.ATI ON 0000 FB LOCATION 0006 LC
SY MB OL.	LABEL 6000 103 103 170 201 201 201 410 900 900	2 11 00 11 10 11 11 11 11 11 11 11 11 11		SYMBOL SYMBOL E
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TORMAS	LABEL 102 102 202 202 203 204 304 304 908 608	8	NZ, NZ)	SYMBOL SYMBOL D
LOCATION DOL 6FC	LGCATEON 001958 001958 001616 002016 002745 002745 002746 002746 002746 002746	003 TE 003 TE 003 B.Z.A. 003 B.Z.A.	SUBROUTINE TAPRO (A.B.C.E.FACT.NI.N.Z.) DIMENSION AINI.MZ.).8 (MZ.MZ.).C(MI.N.Z.).D(10e.10).E(NZ.MZ.) A SUBROUTINE TO PENFORM A-TRANS • B • C MULTIPLICATION RETURN SOLUTION IN E DO 100 1= 1.N.Z. DO 100 1= 1.N.Z. DO 100 K= 1.N.Z. DO 200 K= 1.N.Z. DO 200 M= 1.N.Z. E(1.J.) = D(2.J.) • A(K.I.) • B(K.J.) E(1.J.) = 0.0 DO 200 K= 1.N.Z. E(1.J.) = 0.0 E(1.J.) = 0.0 E(1.J.) = 0.0 E(1.J.) = E(1.J.) • D(E, K) • C(K.J.) • FACT	LCCATION 0000F 0 LDCATION 000108
SUBPROGRAMS CALLED ON SYMBOL SQRF	AAP (ABEL 50 101 221 221 201 201 6500 303 303 303 303 510 650	i	(A.B.C.E.FACT.NI.NZ) B.NZ.NZ) C(MI.NZ) TO PERFORM	SCALAR MAP ON SYMBOL I ARRAY MAP ON SYMBOL
SUBPI LOCATION OOL6F6	LOCATION COLISAC 00118AC 00118AC 00118C 0011C04 0021F6 0021F6 0021F6 0028A4 00289A 00289A 00289A	VERSION 3. 003736 003868 78 877ES	SION A(NI, NZ), 8(NZ, SION A(NI, NZ), 8(NZ, SION A(NI, NZ), 8(NZ, SION A(NI, NZ), 8(NZ, SION A(NI, NZ), 8(NZ, SION A(NI, NZ), 8(NZ, SION A(NI, NZ), 8(NZ, SION A(NI, NZ), 8(NZ, SION A(NI, NZ), 8(NZ, SION A(NI, NZ), 8(NZ,	SCALA 10CATION 0000EC ARRAL 10CATION 000104
SYMBOL	LABEL 475 105 105 200 211 220 230 302 470 470	8 2 2		SAMBOL NZ SAMBOL SAMBOL B
LOCATION 0016F4	LCCATION 00184A 00184A 001858 001628 002178 00244C 002456 002886 002886 002086	IV MODEL 44 II 003664 MEMORY REQUIREMENT HIGHEST SEVERITY EXEC FORTRAN (BCD.)	100	11V MODEL 44 LOCATION 0000E6 0000FC LOCATION 000100
SYMBOL	LABEL 450 6100 1004 180 208 208 219 219 301 301 4616 4616	FORTAN 415 765 707AL	0000 0000 0000 0000 0000 0000 00013	SYMBOL LINING NI FACT OF SYMBOL LINING NI PACT

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COMPILER // SOLPAC	EXEC FORT	MIGHEST SEVERITY CODE MAS O EKEC FORTRAN(BCD, MAP)	
FORTRAN	TV NODEL	EL 44 PS VERSION 3, LEVEL 4 DATE	TE 71321 PAGE 0001
	u	********** OVERLAY C	
2000		SUBROUTINE SOLPAC COMMON / COMM / NUMMP, NUMEL, NLOD, NLIN, MBAND, NUMBLK, NBN, NELIZO)	MBAND, NUMBLK, NBN, MEL (20)
6000		COMPON/RAX	IEC, NRECST, NRECL D, NRI, NR2
\$000		DIMENSION DIAG(1800), AIC	, sile00), xile00)
0000		EQUIVALENCE (X(1),6(1),AJ(1)) REAL® DSUM,DA,DB	
8 000		NAMES AND STATE OF ST	
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	102	1-1-1H!	
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0033		DO 200 J= 188, [8]	
0033	i	AND-J+ (JFREC	
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96.00		IF (INITAL .GT. 0) GO TO 150	
1 600		NACE (JULY) PRECSIONESTE	
2400	1	Z	
4400	130	INITAL=1 CONTINUE	
0042		IF (JFRED .EQ. 3) INITAL	
000		.UR.JFRED.GE.IFRED)	
-		1-# + 1° = %	

					71321 PAGE 0004	
0100 0109 0109 08-08LE(X(1)) 0110 0110 02.0N-08-08 0111 603 CONTINUE 0112 X(J)=3(J)-SNGL(0SUM) 0113 X(J)=X(J)=X(J) FINGL(0SUM)	eQUO CONTINUE eQUO CONTINUE NR4=LR4 CALL DUTIN (1,X ,MRS) SOO CONTINUE Ceeeeeeeeeeee	DO 700 J=1, ML OD NR4 = (J-1)*NRECLD+MR4LOO LR4= NR4 NRD=NRS CALL OUTIN (2, x , MRS) BINRS = KINRS / DIAG(NRD) DO 800 1=1 MRS	0127 0128 0129 0131 0133 0134 0135		MODEL 44 *> VERSION 3. LEVEL 4 DATE NI = I + (JFRED-1)** IF (I - (K-MBAND))***02.805 BO CONTINUE BO DA DELE (B(K)) BO DA DELE (B(K)) BO DA DELE (B(K)) BO DA DELE (B(K)) BO DA DELE (B(K)) BO DA DELE (B(K)) BO DA DELE (B(K)) BO DA DELE (B(K)) BO DA DELE (B(K)) BO DA DELE (B(K)) BO DA DELE (B(K)) BO DA DELE (B(K))	0152 802 CONTINUE 0153 805 CONTINUE 0155 805 CONTINUE 0155 805 CONTINUE 0155 805 CONTINUE 0155 805 CONTINUE 0156 805 CONTINUE 0156 805 015

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	L DCATION 000010	LOCATION 000010	LOCAT 10N	LOCATION	L DC AT TON 0028C4 0028D8	0028EC 002C00 002C14 002C20	LOCATION	LOCATION	LCCATION 0076 EC 00780E 0078 FC 0078 SB 0076 EZ
MACE 0003	SYMBOL	SYMBOL	SYMBOL	SYMBOL	SYMBOL 11		SVMBOL	SYMBOL	LABEL 170 400 206 603 805 700
•	LBCATION 00000C	LDCAT1 ON 000000 0000 20	LOCATION	LOCATION	LOCATION 002 BCG 002804	0028E8 002BFC 002C10 002C24 002C40	LOCATION	LOCATION	L CCATION 00750A 00778A 00778A 00778A 00778A 00778A
	SIZE OCODEC SYMBOL NLIW	SIZE 000024 SYMBUL NR401A NR2	S12E 000004 SYMBDL	SYMBOL	SYMBOL NE I R	INITAL JJ K1 K1 DA	SYMBOL	SYMBOL	LABEL 160 200 203 602 803
13611	CONN / NAP S LOCATION COCCOG	RAK / MAP S LOCATION OCCOOS GOOOLC	DUMP / MAP S LOCATION	L DCATION 000184	LOCA 710N 0028BC 0028DO	0028F4 0028F4 002C0C 002C20	LOCATION	ED LOCATION 0072 A 8	LCCAT1 CM 007634 00771A 00787C 0078D B 007E G
רבאבר א	BLOCK / SYMBOL NLOD NEL	BLOCK / R SYMBOL NR4STF NR1	BLOCK / D	EQUIVALENCE DATA ON SYMBOL AJ	RAP SYHBOL M IL	N1 JR1 LM LMM DSUM	KAP SYMBOL	SUBPROGRAMS CALLED ON SYMBOL SQRT	148 EL 150 202 207 207 900 900
	COMMON LOCATION OCCOOL	COMMON LOCATION 000000 000018	COMMON LOCATION	EQUIVAL LOCATION 000184	SCALAR LOCATION 90.2888 002.8CC	0026E0 002508 002508 002516	ARRAY I LOCATION 004870	SUBPRO- LOCATION 007244	LABEL 10CA TI ON 1002 CO 77 CO 00 00 77 CO 00 00 00 10
3	SV MBOL NUMEL NBN	SYMBOL NR 4L CD NR ECL D	SYMBOL	SYMBOL	SYMBOL NR D I FRED	IA1 142 168 168	SYMBOL A1	SYMBOL	L A 3
	LOCATION 000000 000014	LOCATION 000000 000014	LOCATI DN 000000	LOCATION 000184	LOCATION 002884 0028C8	0028DC 0028F0 002C04 002C18	LOCATION 002C50	LOCATION 007240	BEL LCCATION 101 007510 201 007700 204 007856 100 607832 600 007892 804 007828
	SYMBOL	SYMBOL NR4 NRECST	SYMBOL 1 TEST	S YNG CIL	SYMBOL NRS LR4	0 1	SYMBOL	SYMBOL	LABEL 101 201 204 100 600 600

FORTRAN IV MODEL 44 PS VERSION 3. LEVEL 4 DATE 71321 COMPILER MIGHEST SEVERITY CODE MAS O //DEFL EXEC FORTRAN(BCD,MAP)

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+ STRAIM(1,2)#11 1) 10P,STRAIM, E11 , E22	4 NO 1 J K L 12K K. SHTAU 12, 18X, SHPARGIN	.KQO ,KRO.KSO , STRESS(1,2) 1,4MS.IMT. 10,3,1MS.	.E22 , E11 , E12 , E33 TAPE 2	.E9. 13 GO TO 320 1) OF SAFETY IN EACH LEG & . SF10.5 1	DATE 71321	/(1.0-YMS)00.5 /(1.0-
STRESSI STRESSI CONTINUE CALL HAM IF INLIN IF (ELZ IF (ICN)		MRITE(6,6130) 10P.KPO 1	6140 FORPAT(22K, 9F19.3) 160 CONTINUE 300 CONTINUE NRZ-1.J WRITE(2'NRZ) 1 ,TO ,NQO ,KRO,KSO,E22 1 ,TO ,NHIL	IF (ICHT GT. 1 - AND . 10PT " 3	TORQ = APPLO(7, 12) XOSHR = APPLO(6, 12) YOSHR = APPLO(6, 12) YOSHR = APPLO(6, 12) YOSHR = APPLO(6, 12) YOSHR = APPLO(6, 12) YOSHR = APPLO(6, 12) YOSHR = APPLO(6, 12) YOSHR = APPLO(6, 12) YOSHR = APPLO(6, 12) YOSHR = APPLO(6, 12) YOSHR = APPLO(6, 12)	XYALW = XYLOD (11.0-YMS)*** XMALW = XNOM /(1.0-YMS)*** YMAL = YMOM /(1.0-YMS)*** YMAL = YMOM /(1.0-YMS)*** YMAL = YMOM /(1.0-YMS)*** YMAL = YMOM /(1.0-YMS)*** YMAL = YMOM /(1.0-YMS)*** YMASAL = YMSHR(1.0-YMS)*** YMSAL = YMSHR(1.0-YMS)*** YMSAL * YMSHR(1.0-YMS)*** ** ** ** ** ** ** ** ** *
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	F15. 5, 340 S1	F15.5, 316	SIZE 0000&C Symbol NLIN	SIZE 0000DC SYMBOL FXC SYMBOL THETA 10PT 1CMT
	DIRECTION 6.	· F	DATE 71321 COMN / MAP LOCATION 000006 00001C	ONE / NAP LOCATION ODOOS4 LOCATION ODOOS2 OOOOS6 OOOOOS
11.671°51DE DE 1 60 TO 340	Ke3 - 2 T - 2 T COULUS IN X) 60 TO 360	47-2) Ke3 - 1 LG7 DOULUS IN Y	ON BLOCK / SYMBOL MLOD NEL	SYMBOL FYT SYMBOL SYMBOL THE SYMBOL THE DUM NY ITET
= ((NX-1)*Y2LGT + Y1LGT)*SIDE NATE STIFFWESSES NATE STIFFWESSES NATE STIFFWESSES NATE STIFFWESSES NATE STIFFWESSES NATE STIFFWESSES = XLGAD/(HTEXLGT) (NX-1) (NX-1) (NX-2) (NY-2*1)*(4*NY-1)*3 (NY-2*1)*(4*NY-1)*3 (NY-2*1)*(4*NY-1)*3 (NY-2*1)*(4*NY-1)*3	6 K = 1.NUMNP DO(K) = EC. J1) N1 = Ke3 - 2 DO(K) = EC. J2) N2 = Ke3 - 2 DO(K) = EC. J2) N2 = Ke3 - 2 EC. SCO. J2) N2 = Ke3 - 2 = 578.57 KN (6.6200) XMOO I (//, 5%, 26H MOOULUS IN X ESTYLOAD = LT001) GO TO 360 ET. EQ. J1 GO TO 350 (NXe(NY-1)+1) NXENY (26NY-1) (26NY-1) (26NY-1)	((400Y-1)0(NY-1)+20NY-2) 6 K = 1,NUNNP 6 K = 1,NUNNP DO(K) .EG. J1) N1 = K03 DO(K) .EG. J2) N2 = K03 = (-8(N1)+8(N2)) / XLGT = (-8(N1)+8(N1)+8(N2)) / XLGT = (-8(N1)+8(N1)+8(N2)) / XLGT = (-8(N1)+8(N1)+8(N1)+8(N1)+8(N1)+8(N1)+8(N1)+8(N1)+8(N1)+8(N	COMMON LOCATION COCOCO COCOCOCOCOCOCOCOCOCOCOCOCOCOCO	COMMON 000050 000054 COMMON COCATION 000006 000008 000000
XLGT = ((LKX-1)*YZ YLST = (FLOAT(NY) CALCULATE STIFFNE IF (ABS:KLOAD) .L STRS = XLOAD/(HTO IF (ITET .EQ. 1) G JJ = (NX-1) VLT = YLGT GO TO 335 JJ = (NYZ*E)**(+0NY- JJ = (NYZ	ELTERNITE P.	12 - 15 C 15 C 15 C 15 C 15 C 15 C 15 C 15	44 PS SYMBOL NUMEL NBM	SYMBOL FXT CMS SYMBOL HT HAT MX
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MODEL 44 PS VERSION 3. LEVEL 4 DATE 71321	SUBROUTINE BURL FBRL) COMMON / MAIL / E(4,5),FXT(5),FYT(5),FXC(5),FYC(5),SSS(5),CMS(10) COMMON / ONE / SIDE,MT,THK(5),TMETA,YIOFF,YZOFF,TMT,DUM,TC(11) 1 ,XC(11),HEO(12),MX,NY,IOPT,ITYP,IFACE,IMOLE,ITET,ICMT,NNEW(4) DIMENSIGN FBRL(5),D(3),XL(5) N = 3		IF (1-3) 28C,28O,277 27 XM: ~2(3.14) 59/XL(1)) 002-0(1011) 00(2)) 00.5 0 0 13)) 60 TO 345 280 PHM ~ 3.141590H T/SIDE GAM ~ 4(6.3,1) 0 XMU E(2,1) + E(4,1) 0 PHM ~ D13) /D (2) 0 PHM I COT ~ 0	XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX	292 IF (140) 293.305.293 293 MFT	TTE 16. TURN 1
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GLOSSARY

	·
A	- Merged stiffness matrix
Ā	- Triangularized stiffness matrix
В	 Matrix relating plate strains to plate nodal deflections
C ₁₁ ,C ₂₂ , etc	- Plate elasticity matrix
D ₁ ,D ₂ ,D ₃	- Plate bending stiffnesses
Ex	- Plate elastic modulus in fiber direction
Еу	 Plate elastic modulus transverse to fiber direction
FXT	 Plate tension allowable stress in fiber direction
F _{XC}	- Plate compression allowable stress in fiber direction
F _{YT}	- Plate tension allowable stress transverse to fiber direction
F _{YC}	- Plate compression allowable stress transverse to fiber direction
GXY	- Plate shear modulus
r ^X .	- Number of Vertical legs on X face
L _Y	- Number of Skew B legs on Y face
^N x	- Plate buckling load in X direction (lb/in.)
P _X	 Vertex nodal load for uniform load in X direction
P _Y	 Vertex nodal load for uniform load in Y direction
P _{.XY}	 Vertex nodal load for uniform load in XY direction
P _{MX}	 Vertex nodal load for uniform moment in X direction

P_{MY} - Vertex nodal load for uniform moment in Y direction

P_{MXY} - Vertex nodal load for uniform moment in XY direction

Pz - Vertex nodal load in Z direction

APPENDIX I ADDITIONAL GENERATION EXAMPLES

Additional examples of flat plate model generation are included in this appendix to clarify the examples given in the body of the report. The nodal numbering systems used for a 6 by 6 and a 10 x 6 true Tetra-core plate are shown in Figures 29 and 30, respectively. The nodal numbering systems used for a 4 x 6 and an 8 x 4 truncated Tetra-core plate are shown in Figures 31 and 32. Plate numbering systems used for the Vertical, Skew A, and Skew B legs in the truncated Tetra-core flat plate are shown in Figures 33, 34, and 35. The addition plates added to Skew A and Skew B legs to complete the cylinder model are shown in Figures 36 and 37. The nodal loads generated for a true Tetra-core flat plate model for each load type are shown in Figures 38, 39, 40, 41, 42, 43, 44, and 45. The nodal loads generated for a truncated Tetra-core flat plate model are shown in Figures 46, 47, 48, 49, and 50.

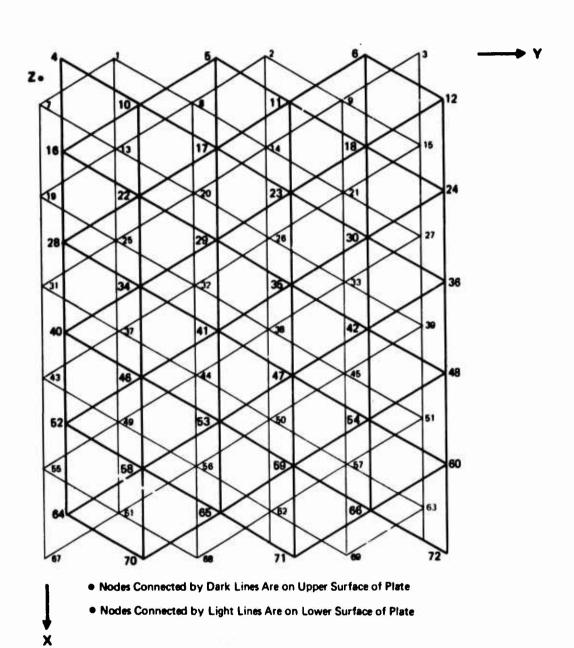
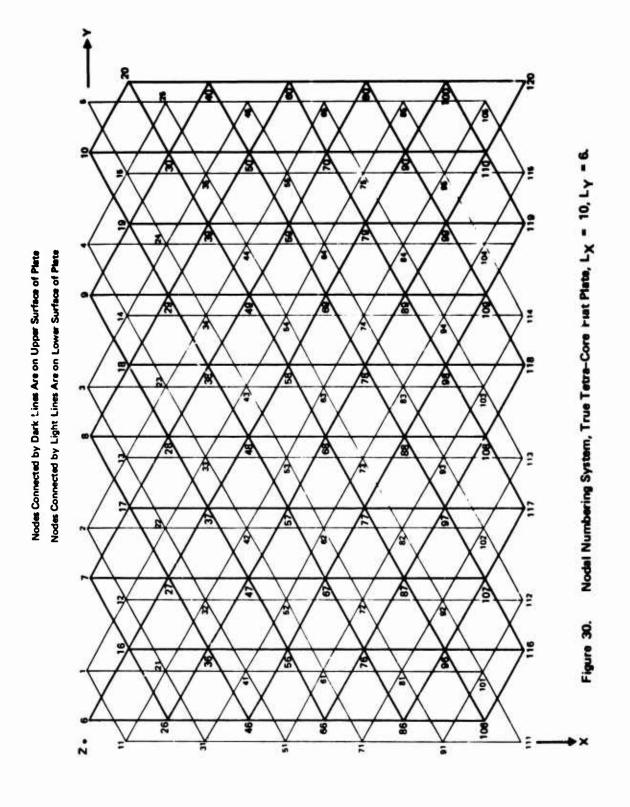


Figure 29. Nodal Numbering System, True Tetra-Core Flat Plate, $L_X = 6$, $L_Y = 6$.



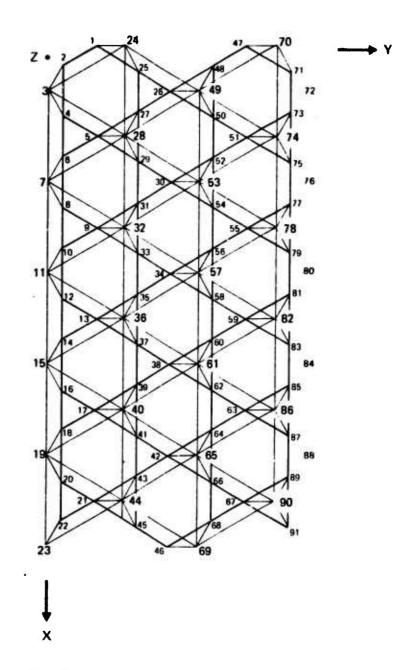


Figure 31. Nodal Numbering System, Truncated Tetra-Core Flat Plate, $L_X = 4$, $L_Y = 6$.

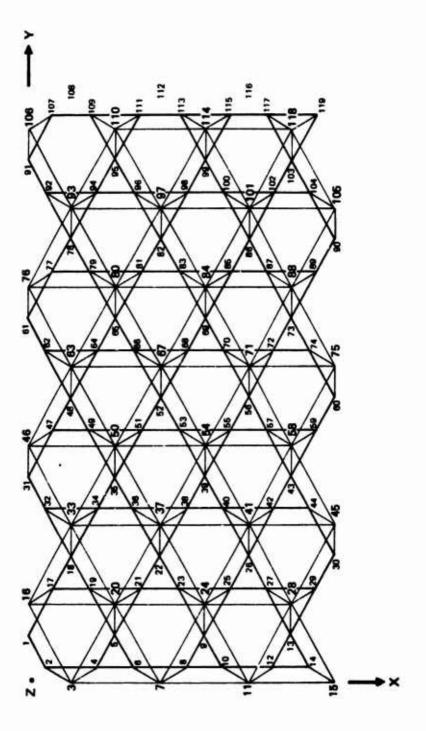


Figure 32. Nodal Numbering System , Truncated Tetra-Core Flat Plate, $L_X = 8, L_Y = 4$.

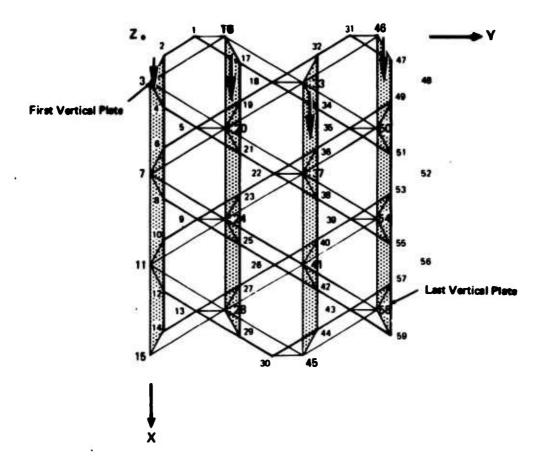


Figure 33. Vertical Leg Plate Numbering System Truncated Tetra-Core, $L_X = 4$, $L_Y = 4$.

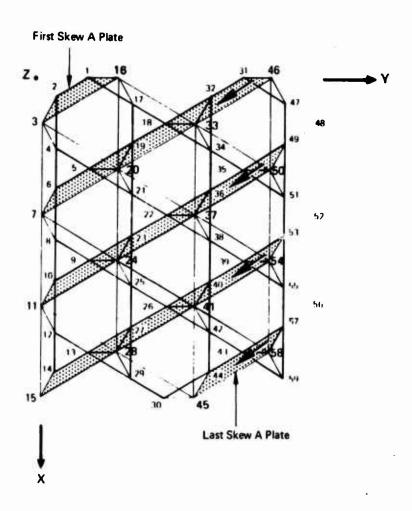


Figure 34. Skew A Leg Plate Numbering System Truncated Tetra-Core, $L_X = 4$, $L_Y = 4$.

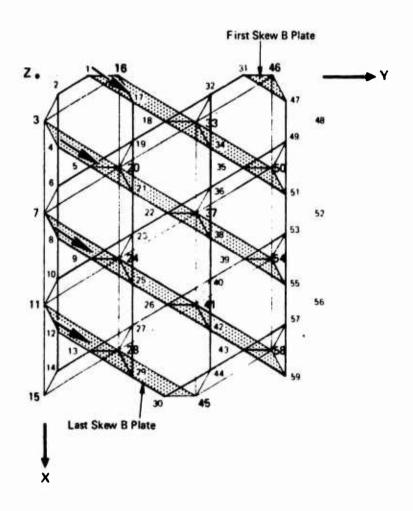


Figure 36. Skew B Leg Plete Numbering System Truncated Tetra-Core, $L_X = 4$, $L_Y = 4$.

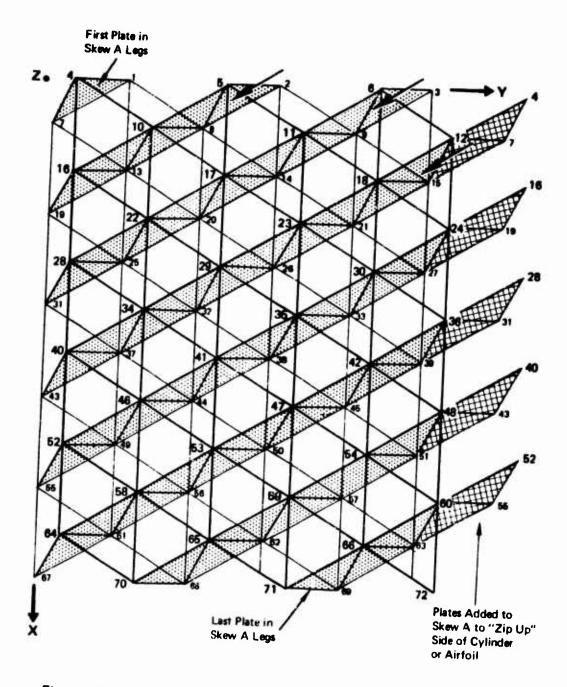


Figure 36. Skew A Plate Numbering System, True Tetra-Core Flat Plate, $L_X = 6$, $L_Y = 6$.

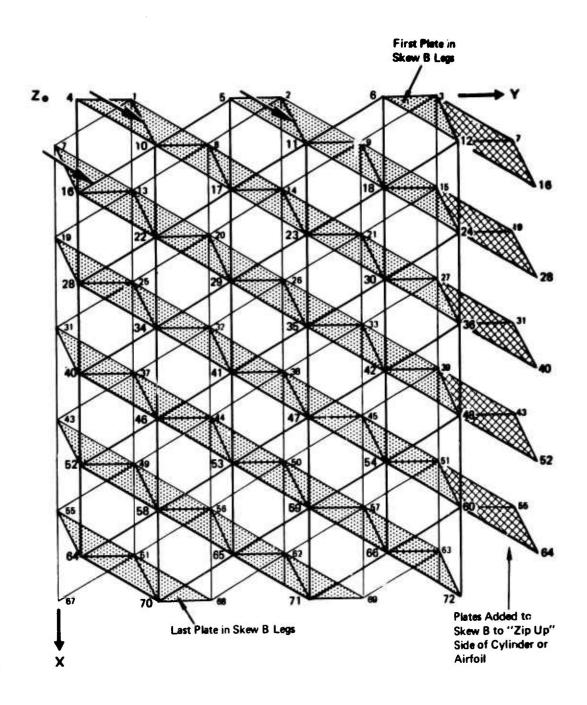


Figure 37. Skew B Plate Numbering System, True Tetra-Core Flat Plate, $L_X = 6$, $L_Y = 6$.

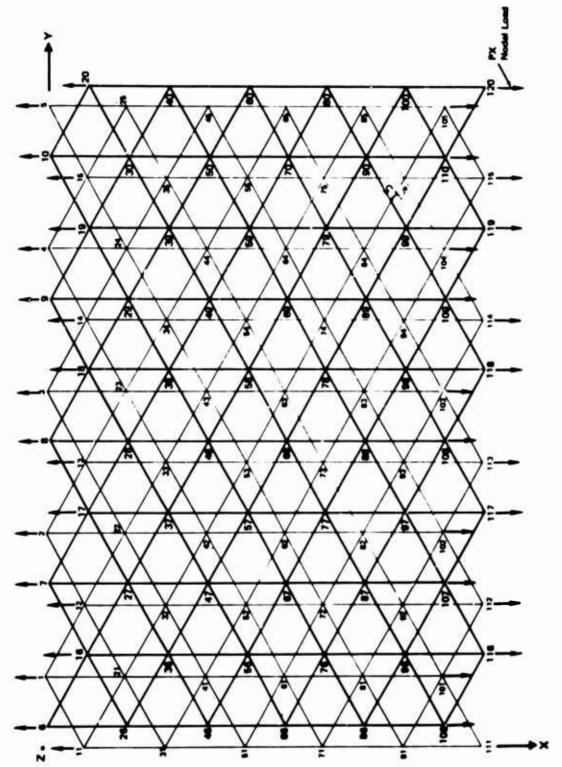
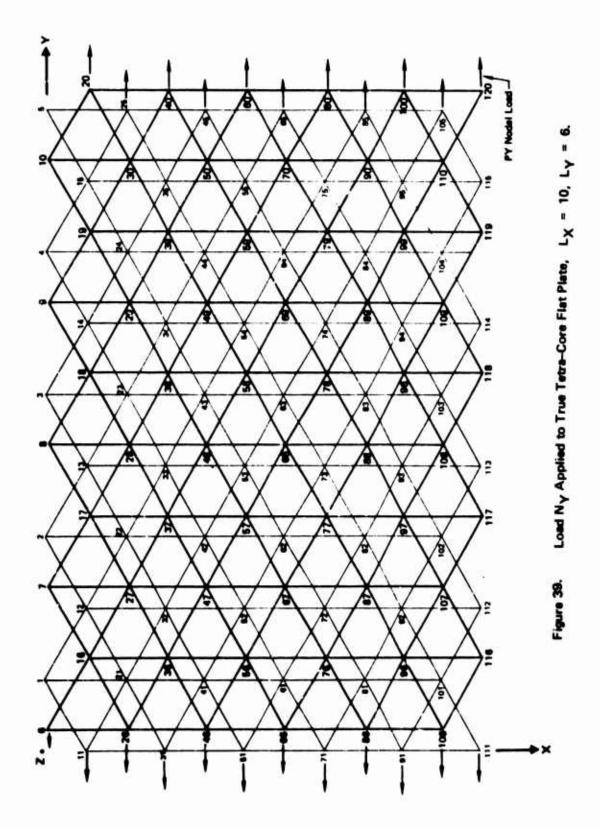
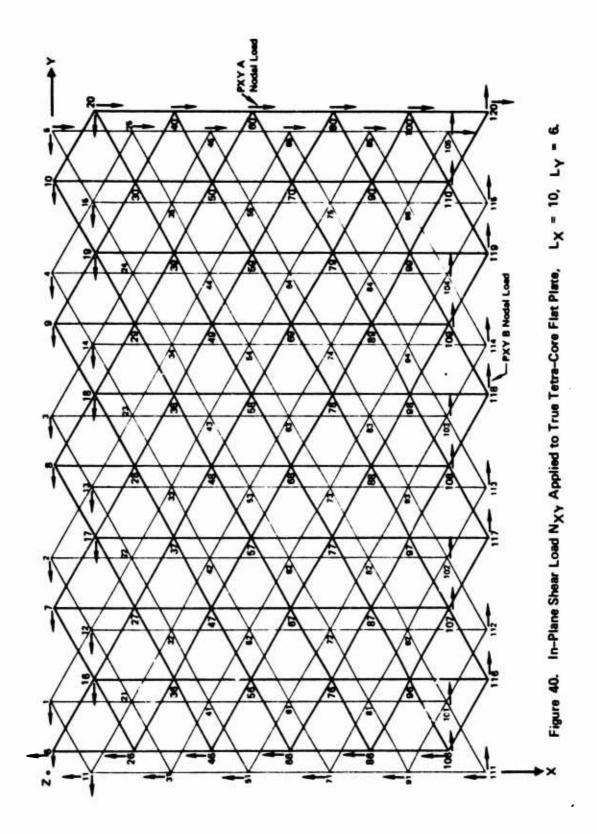
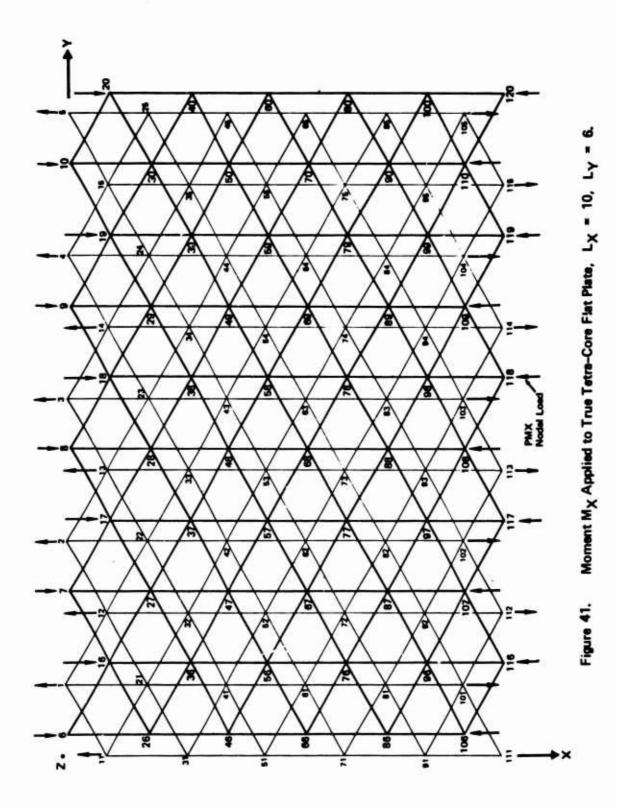
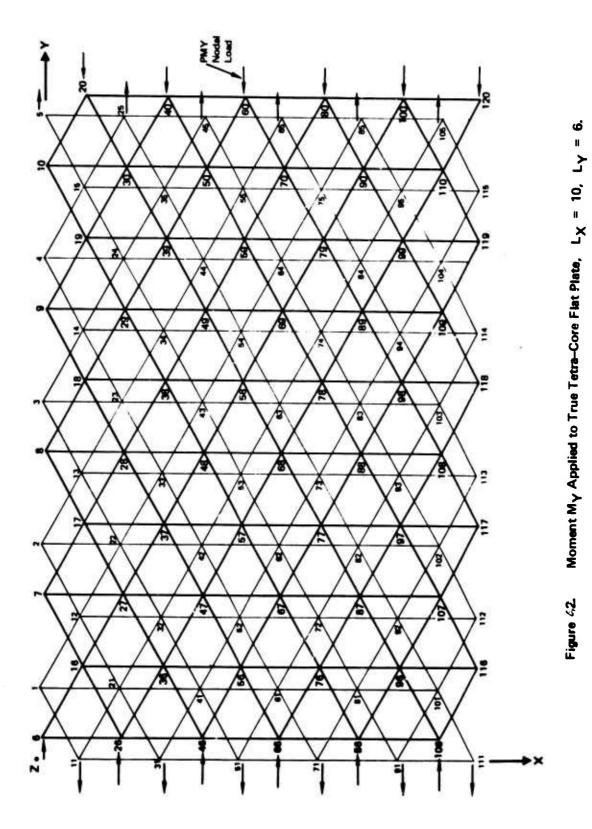


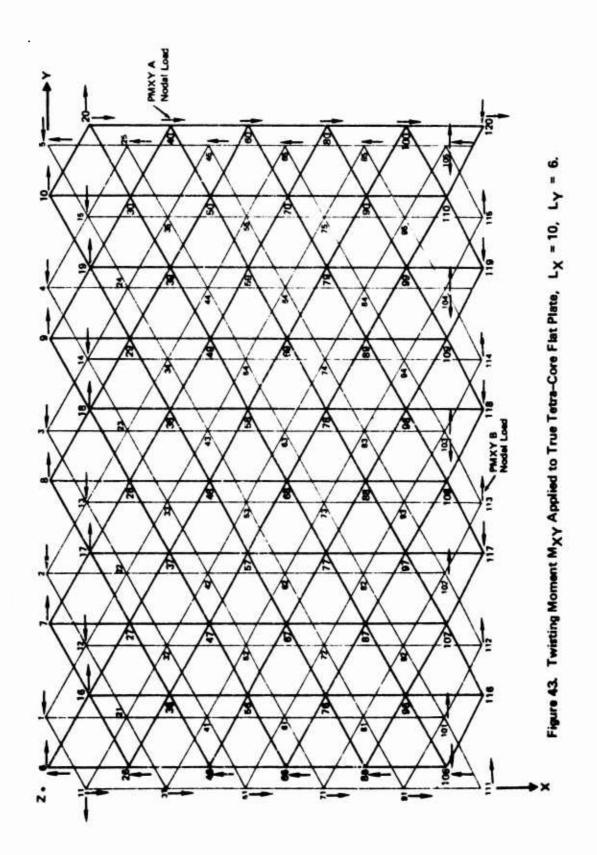
Figure 38. Load N_X Applied to True Tetra-Core Flat Plate, L_X = 10, L_Y = 6.











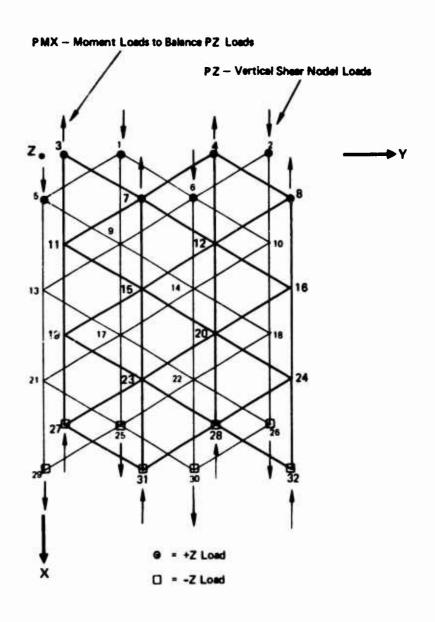


Figure 44. Vertical Sheer Load XOSHR Applied to Tetra-Core Flat Plate, $L_X = 4$, $L_Y = 4$.

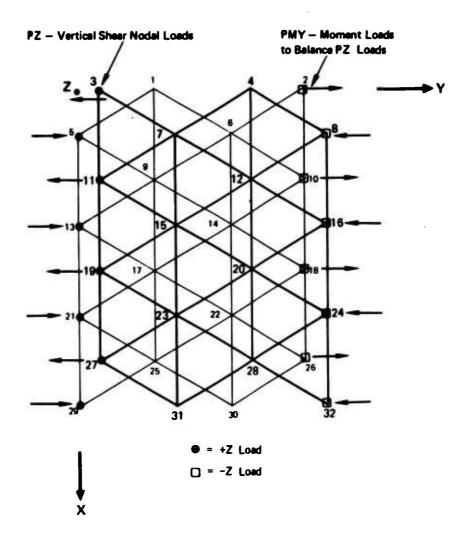


Figure 45. Vertical Shear Load YOSHR Applied to Tetra-Core Flat Plate, $L_X = 4$, $L_Y = 4$.

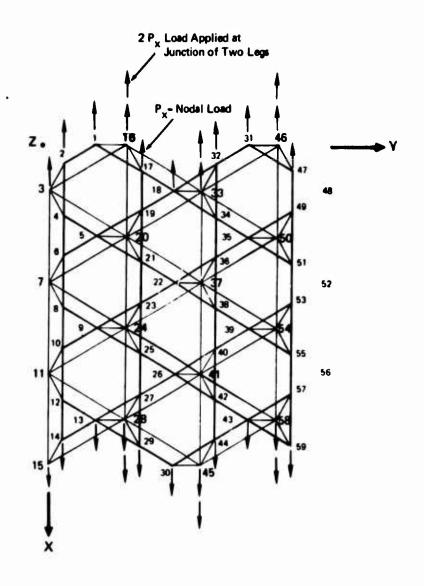


Figure 46. Load N_X Applied to Truncated Tetra-Core Flat Plate, $L_X = 4$, $L_Y = 4$.

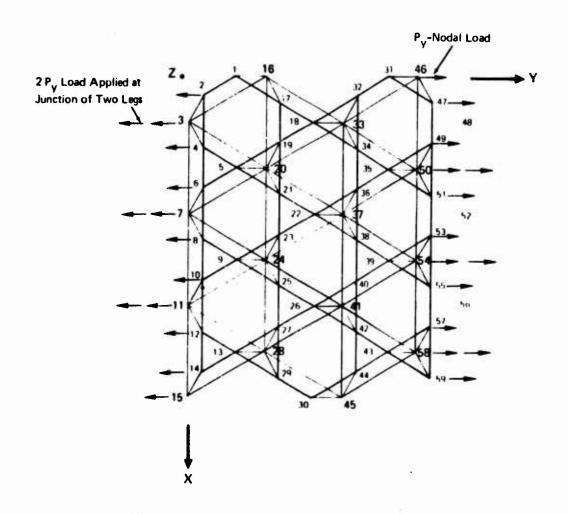


Figure 47. Load N_Y Applied to Truncated Tetra-Core Flat Plate, $L_X = 4$, $L_Y = 4$.

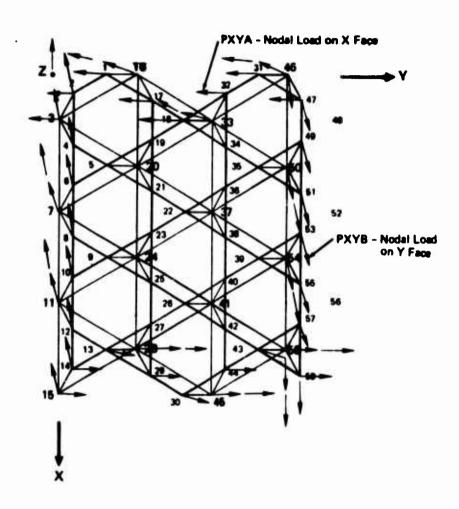


Figure 48. In-Plane Sheer Load N_{XY} Applied to Truncated Tetra-Core Flat Plate, $L_X = 4$, $L_Y = 4$.

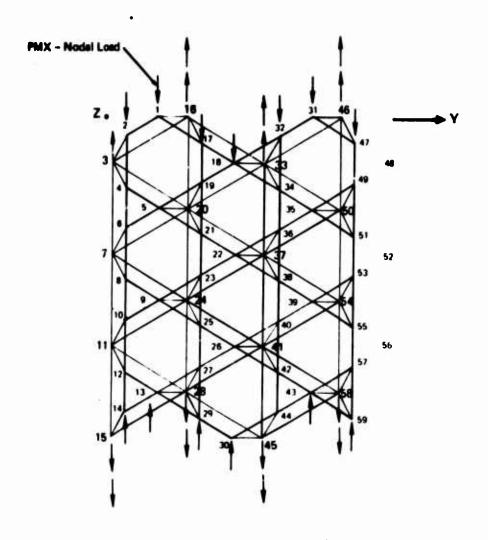


Figure 49. Moment M_X Applied to Truncated Tetra-Core Flat Plate, $L_X = 4$, $L_Y = 4$.

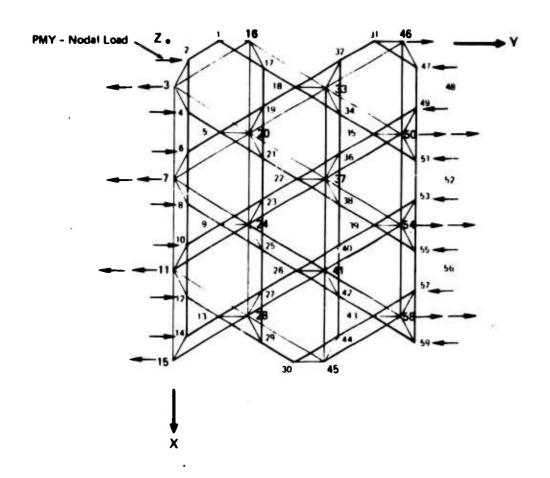


Figure 50. Moment My Applied to Truncated Tetra-Core Flat Plate, $L_X = 4$, $L_Y = 4$.

APPENDIX II EXAMPLE OF COMPUTER PROGRAM OUTPUTS

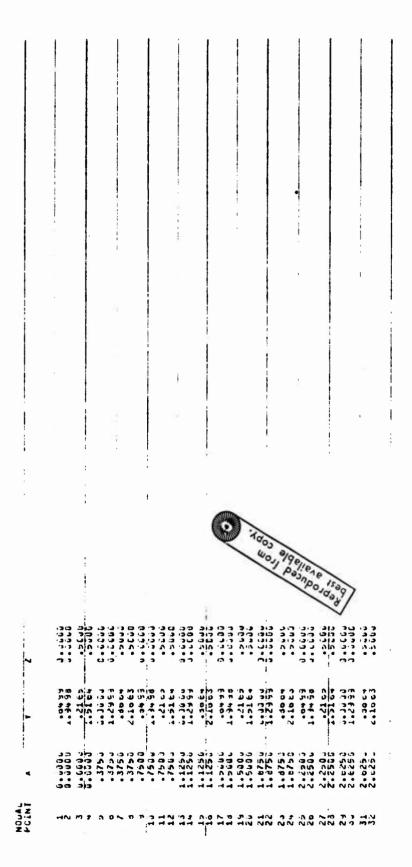
An example of the output from an optimization run is given (Figure 51). Stresses and deflections for the input configuration are printed. Then the optimization steps are printed. Finally, stresses and deflections for the optimized section are printed. Element stresses are printed out of sequence in this example, although they are in sequence in the latest version of the program.

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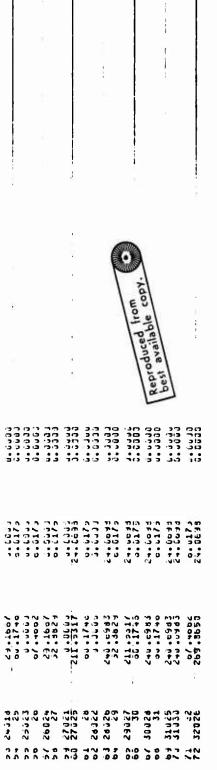
AL LOWARLE STRESSES Figure 51. Output from Optimization Run.

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OPTIMIZATION STEPS

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TATE CONTRACTOR CONTRA	14 H 12 3. U. 3. U. 3. U. 3. U. HARUINIS	IN = 12 0000 MARCIN(1) ESCENDÍNG 2500	######################################	# = 12 # = 12 3000 3000	MARCIN(I) = NEW CELLOR	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	2	IK = 2 MI = . uduu PAKCINII = -1.33244	1 = 3

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